

## **2. GENERAL DESIGN AND LOCATION FEATURES**

The design of a bridge typically takes place in two major phases of work: preliminary design and final design. During preliminary design, the structure type, the foundation type, the aesthetics, and the primary geometry for the bridge are determined. During final design, specific details for all elements of the bridge are developed and presented in the plan set. These details include material descriptions, quantities, and geometric information. Final plan sets are typically assembled in an order that roughly follows the order of construction, from the ground up.

This section of the manual contains a large amount of information useful for the preparation and assembly of plans for a project. To facilitate the production of plans and standardize the content of bridge plan sets, the Bridge Office has developed special provisions, standard bridge details, standard plans, standard plan notes, and standard pay items.

Guidance for the design of specific structural elements (e.g. beams, abutments, piers, etc.) is provided elsewhere in the manual.

### **2.1 Geometrics**

#### **Definitions**

For discussion of bridge geometrics in this section, roadways are classified as Mainline Highways, Ramps, Local Roads, and Local Streets. Each of these four groups is further classified under either Urban or Rural Design.

The following definitions apply:

- Mainline Highways – Roadways that carry through traffic lanes for freeways, expressways, and primary and secondary highways.
- Local Roads – Rural roads off the state trunk highway system.
- Local Streets – Urban roads off the state trunk highway system.
- Ramps – Segments of roadway connecting two or more legs at an interchange.
- Urban Design – Roadways with curbs on the right and/or left sides.
- Rural Design – Roadways without curbs.
- Median Width – The distance between the inside edges of opposing through traffic lanes.
- Auxiliary Lane – A lane adjoining a through traffic lane for a purpose supplementary to through traffic movement such as truck climbing, weaving, speed change or turning.

#### **2.1.1 Bridge Geometrics**

##### **General Criteria**

The width of the bridge deck and the typical section at the bridge undercrossing are determined by the classification and geometrics of the approaching roadway, together with appropriate design considerations for

shoulder needs. The geometrics of the approaching roadway are to be carried over and under the bridge to the maximum extent practicable.

Bridge width requirements are a function of the lane and shoulder widths of the approaching roadway, together with assessment of pedestrian and bicycle needs, multimodal requirements, user safety requirements, drainage requirements, staging, and other project specific considerations such as snow storage and emergency vehicle access. The determination of the appropriate width for each project requires study of specific project needs. Detailed decision documentation is required by the Roadway Designer during the preliminary design phase and must be coordinated with the Preliminary Bridge Plans Engineer. Bridge shoulder and lane widths should be included with project design element documentation in the District project design memo, including informal design exceptions as necessary.

The discussion of geometric details included in this section describes bridge deck geometrics separately from bridge undercrossing geometrics.

#### **Application of Standards**

Unless stated otherwise, the geometrics discussed in the following articles apply specifically to new work. However, use of these geometrics is also highly desirable when upgrading or widening existing facilities and should be incorporated in those situations also. For bridge repair projects, see the *Bridge Preservation and Improvement Guidelines*, found on the MnDOT Bridge Office web site, for more information. Bridge deck geometrics on the local road system must comply with *State Aid for Local Transportation Operations Rules*, Chapter 8820. Note that an exception to this applies to local road system bridges at an interchange with a trunk highway, which must be designed to trunk highway standards between the ramp terminals on the local road.

#### **Responsibility**

The Preliminary Bridge Plans Engineer will be responsible for assuring that the geometric standards in this section are followed. Where a deviation from the standard is necessary, a written description of the deviation shall be prepared by the Preliminary Bridge Plans Engineer and submitted to the State Bridge Engineer for approval prior to submitting the Preliminary Bridge Plan for signature.

#### **2.1.2 Bridge Deck Requirements**

##### **Bridge Width Criteria**

Roadway cross sections that approach bridges will normally provide a clear zone outside the travel lanes as a recovery area for out-of-control

vehicles. It is typically not economical or practical to carry these clear zones across bridges. Therefore, crash-tested bridge railing is required across the bridge, and guardrail protection is typically required in the approach area.

Roadway shoulder and bridge shoulder width standards have been developed to allow project designers more flexibility, providing them greater latitude to address specific project requirements. For the majority of bridges, the bridge width will match the approach roadway width. For longer and/or more complex bridges, a risk assessment of non-standard width options will be performed to determine the appropriate bridge width. Refer to the document *Performance-Based Practical Design Process and Design Guidance*, found as an attachment to Technical Memorandum No. 18-09-TS-07. (<http://www.dot.state.mn.us/pbpd/design-guidance.html>)

Detailed design decision documentation should include a checklist leading to the selected bridge width over, and portal width under, for the project and must include consideration of the following functions of the shoulder:

- Recovery area to regain control of a vehicle.
- Emergency parking area for stalled vehicles and escape route for stranded motorists.
- Passageway for bicycles and pedestrians.
- Passageway for emergency vehicles.
- Parking area for bridge maintenance and inspection vehicles (working area for under-bridge inspection vehicle and lane closure requirements).
- Temporary traffic lane during deck repairs or overlay construction.
- Area for deck drainage and snow storage.
- Accommodation for passing of wide oversize loads, especially farm machinery.
- Escape area to avoid a head-on collision with an oncoming passing vehicle on a two-lane highway.
- Designated bus shoulder lane.
- Staging needs during construction.

For local roads and streets, bridge widths are given in the *State Aid Manual*, Section 5-892.210 and the *State Aid Operations Rules*, Chapter 8820. Note that local road system bridges at an interchange with a trunk highway must be designed to trunk highway standards between the ramp terminals on the local road.

**Cross Slopes on Bridges**

- 1) Use a cross slope on the bridge traffic lanes that is the same as the approaching roadway lanes, normally 0.02 ft/ft. The shoulder on a bridge may continue at the adjacent lane cross slope or, if better drainage is desired, may be 0.005 ft/ft greater than the adjacent lane. If a shoulder functions as a pedestrian access route, cross slopes must not exceed 0.02 ft/ft to be ADA-compliant. When the bridge deck is superelevated, provide the same slopes for the shoulders as the adjacent bridge traffic lanes. The 0.005 ft/ft maximum cross slope change between adjacent lanes and shoulders is determined for constructability by limiting the need for atypical detailing such as special bar bends in the deck. Also note that the greater the change in cross slope, the more difficult it is to remove snow to bare pavement. Changes in cross slope between adjacent lanes and shoulders that are greater than 0.005 ft/ft will be considered where steeper slopes will reduce the number of deck drains on the bridge, but must be approved by the Preliminary Bridge Plans Engineer. Note that the effects of a changing cross slope are magnified on curved alignments and require additional consideration and adjustment of stools, seat elevations, and resulting encroachment on vertical clearances.

Keep superelevation transitions off bridges. In instances where they are unavoidable, it is preferable for ease of deck placement to maintain a straight line across the deck at all locations, because it allows a straight screed between paving rails placed at both sides of the deck. Locate begin and end points of transition breaks at piers.

- 2) Provide ramp cross slopes that are uniform between the bridge curbs.

**Bridge Median**

On divided highways with a separate bridge for each roadway, the openings between bridges must be a minimum of 8'-0" wide if access for bridge inspection vehicles is required.

Use longitudinal joints along the median of bridges only on bridge roadways wider than about 100 feet or for other special cases. By eliminating this joint on bridges with medians, simpler detailing and simpler construction can be used.

**Sidepaths (Shared-Use Paths) and Sidewalks on Bridges**

Sidepaths are shared-use paths parallel to the roadway that are physically separated from motor vehicle traffic. Providing a sidepath on a bridge is an option to accommodate both pedestrian and bicycle traffic. Sidewalks are similar but are intended to accommodate pedestrian traffic only.

The width of bridge sidepaths and sidewalks are highly dependent on their context (i.e., factors such as land use, user type, expected volume, state and local non-motorized plans, network connections, trip attractions, overlooks, future growth, and bridge length).

When including pedestrian and/or bicycle traffic on a bridge, note that safety, accommodation, and cost must be balanced for all users throughout the roadway cross section. This includes balancing the widths of lanes, shoulders, sidepaths, and sidewalks, particularly in constrained cross-sections.

The AASHTO *Guide for the Development of Bicycle Facilities* (GDBF) recommends a minimum two-way shared-use path paved width of 10 feet.

The *Proposed Guidelines for Pedestrian Facilities in the Public Right of Way* (PROWAG) requires a continuous minimum clear public access route (PAR) width of 4 feet and a minimum clear PAR width of 5 feet at intervals of 200 feet to allow for passing.

On bridges, MnDOT also includes a buffer width added on each side of the sidepath/sidewalk in order to protect users from vertical barriers and edge of sidepath/sidewalk drop-offs.

MnDOT also has additional width criteria based on the maintenance accessible route (MAR). The MAR width requirements allow maintenance forces access for snow and ice maintenance of the sidepath/sidewalk. The MAR width is dependent on the agency responsible for snow and ice maintenance and the equipment they use. The minimum width for the MAR is 6 feet between obstructions (sometimes more if achievable).

Use the following guidance for determination of bridge sidepath/sidewalk widths. For local bridges, also refer to *State Aid Operation Rules*, Chapter 8820.

#### 1) New vehicular bridges

Best practice is to provide continuity by matching the measured width of the approach sidepath/sidewalk and adding a 1 foot buffer width on each side. See Figure 2.1.2.1. For approach sidepaths/sidewalks that are located immediately behind a curb, the approach width is measured to the back side of the curb. Integral brush curbs (maximum of 2 inches wide x 6 inches high) may be included in the clear width dimension where the total width is greater than 10 feet. For approach sidepaths not meeting the AASHTO GDBF recommended minimum width of 10 feet, consult state or local plans and/or the appropriate trail

authority to identify the future intent and feasibility of providing a greater approach sidepath width. Consult with functional group experts as necessary.

Total bridge sidepath/sidewalk widths greater than best practice or greater than 12 feet require consultation with the state or local authority and/or the appropriate trail authority to identify the need for additional width. The District and/or local authority must document the need for and feasibility of providing this width (plan, cross section, letter, user volume, etc.). Total widths beyond 12 feet require concurrence from functional group experts and discussion to determine whether municipal cost participation is necessary.

The minimum total bridge sidepath width for new vehicular bridges is 10 feet, which is based on an 8 foot approach sidepath (two times the 4 foot PAR width) plus a 1 foot buffer width on each side. Consideration may be given to a minimum total bridge sidepath width less than 10 feet when the approach sidepath width is less than 8 feet and/or there is concurrence from functional group experts.

For new vehicular bridges that accommodate pedestrian traffic only, the minimum total bridge sidewalk width is 7 feet, which is based on the 5 foot PAR width for passing plus a buffer width of 1 foot on each side.

The total bridge sidepath/sidewalk width is defined as the minimum clear width measured from the sidepath/sidewalk side of the curb/barrier/parapet/railing to the sidepath/sidewalk side of the opposite curb/barrier/parapet/railing. For situations where there is no barrier/parapet on the traffic side of the sidepath/sidewalk (raised sidepath/sidewalk), the measurement is to the top outside edge of the sidepath/sidewalk. There, the location of the top outside edge of the sidepath/sidewalk is defined as 1 inch from the gutter line (based on 6 inch curb height  $\times$  0.125 slope = 0.75 inches, rounded up to 1 inch). Integral brush curbs (maximum of 2 inches wide  $\times$  6 inches high) may be included in the clear width dimension where the total sidepath/sidewalk width exceeds 10 feet. See Figure 2.1.2.1.

## 2) New pedestrian bridges

For new pedestrian bridges that carry both pedestrians and bicycle traffic, follow the guidance given in 1) above.

For new pedestrian bridges carrying pedestrians only (note that this is a rare occurrence), the minimum total bridge walkway width is 8 feet

per the requirements of AASHTO's *A Policy on Geometric Design of Highways and Streets*.

3) Bridge repair projects

Where possible, follow the guidance given in 1) above for bridge repair projects.

On bridge repair projects with constrained cross-sections, the minimum total bridge sidepath width is 8 feet.

On bridge repair projects with constrained cross-sections that accommodate pedestrian traffic only, the minimum total bridge sidewalk width is 5 feet. Consideration may be given to a minimum total width of no less than 4 feet where constrained bridge cross-sections are less than 200 feet long and there is concurrence from functional group experts. See Figure 2.1.2.1. Note that these minimums are less than the MAR minimum width of 6 feet, which would then require a special plan for snow and ice maintenance.

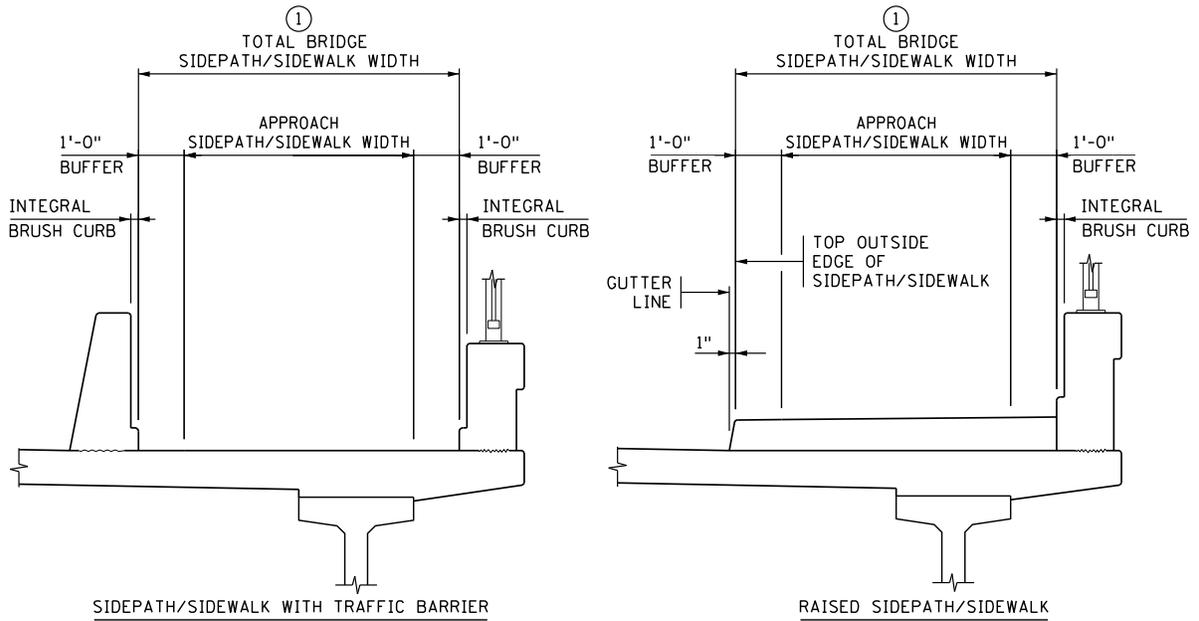
AASHTO defines high speed as design speeds 50 mph and greater, and low speed as design speeds 45 mph and less. Safety for both vehicles and pedestrians/bicycles must be considered when determining separation requirements on a bridge. A separation railing between the roadway and sidepath/sidewalk can provide protection to users of the sidepath/sidewalk, but also can reduce sight distance and present a possible hazard to vehicle traffic. Functional group experts within MnDOT have agreed on the following guidance for determining when to use a separation railing:

- For design speeds of 45 mph and greater, a separation railing is always required.
- For a design speed of 40 mph, consider the following when determining whether separation is needed:
  - Adjacent land uses, such as the pedestrian destinations described in [Minnesota Walks](#).
  - Amount of available space approaching the bridge (length and width) for the appropriate barrier end treatment.
  - Proximity of intersections to the bridge and whether intersection sight distance will be affected by inclusion of barrier and guardrail.
  - Volume of pedestrians, bicyclists, passenger cars, and heavy vehicles that use the bridge.
  - Actual motor vehicle operating speed compared to the design speed.
  - Horizontal alignment and location of the sidepath/sidewalk relative to a curve (e.g. – inside or outside of a curve).

- Regional significance of sidepath/sidewalk connection.
- For design speeds of 35 mph or less, use a raised sidepath/sidewalk without a separation railing.

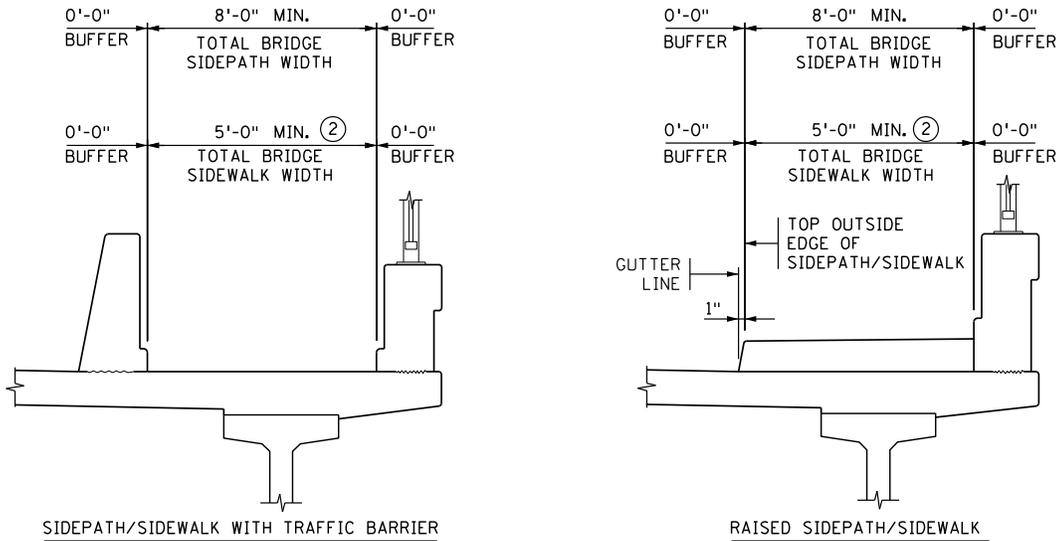
Separation railings must meet the MASH crash test level and traffic railing height requirements found in BDM Article 13.2.1. In addition, a pedestrian/bicycle railing is required on the outside edge of the sidepath/sidewalk. When a separation railing is provided, use the bridge slab for the sidepath/sidewalk. When a separation railing is not used, provide a raised sidepath/sidewalk with a 6 inch curb height measured at the gutter line. Advise the road plans designer to provide for any necessary sidepath/sidewalk ramping off the bridge.

The minimum cross slope for sidepaths/sidewalks is 0.01 ft./ft.



① FOR TOTAL WIDTHS > 10'-0", INTEGRAL BRUSH CURB (MAX. 2" WIDE x 6" HIGH) MAY BE INCLUDED IN TOTAL WIDTH.

SIDEPATH/SIDEWALK WIDTH FOR BRIDGES



② CONSIDERATION MAY BE GIVEN TO A MINIMUM TOTAL WIDTH OF 4'-0" ON CONSTRAINED BRIDGE CROSS-SECTIONS < 200 FT. LONG WITH CONCURRENCE FROM FUNCTIONAL EXPERTS.

SIDEPATH/SIDEWALK  
MINIMUM WIDTH FOR BRIDGE REPAIR PROJECTS  
FOR CONSTRAINED CROSS-SECTIONS

**Figure 2.1.2.1**

### Protective Barriers at Bridge Approaches

The ends of bridge barriers must be protected from being impacted (except on low speed roads such as city streets). For design speeds over 40 mph, a crash tested guardrail transition is required.

Refer to *State Aid Operation Rules*, Chapter 8820 for guardrail requirements on local bridges. Note that local road system bridges at an interchange with a trunk highway must be designed to trunk highway standards between the ramp terminals on the local road.

### 2.1.3 Bridge Undercrossing Geometrics

#### General Criteria for Lateral Clearance

Bridge undercrossing geometrics must rationalize safety requirements with costs and physical controls such as span length and permissible depth of structure. The following guidelines apply in establishing these geometrics:

##### 1) Safety

Piers, abutments, inslopes and unpaved back slopes steeper than 1:3, and guardrails can all be crash hazards to an out-of-control vehicle.

Historically at bridge undercrossings, it was considered best practice to provide a clear zone recovery area beside the roadway, free from these hazards. The clear zone was determined using the MnDOT *Road Design Manual*, Section 4-6.04 and was based on the roadway curvature, design speed, ADT, and ground slope. For the area under bridges a practical maximum clear zone of 30 feet was used as permitted in the *2011 AASHTO Roadside Design Guide*, Table 3-1, based on consistent use and satisfactory performance.

Acceptance of the Performance-Based Practical Design (PBPD) philosophy led to reevaluation of the risk of not meeting the clear zone requirements at bridge undercrossings. This evaluation resulted in new guidance for determination of the appropriate clear zone, which is found in the document, *Performance-Based Practical Design Process and Design Guidance*:

<http://www.dot.state.mn.us/pbpd/design-guidance.html>

In addition, Technical Memorandum No. 21-05-TS-04 was created, which contains the current PBPD guidance for determining the bridge undercrossing width for MnDOT projects:

<https://techmemos.dot.state.mn.us/techmemo.aspx>

Where drainage must be carried adjacent to the roadway passing under a bridge, either a culvert must be provided at the approach fill or the ditch section must be carried through at the toe of the bridge approach

fill. The use of a culvert will often permit more desirable bridge geometrics, but the culvert openings can also introduce a roadside hazard, requiring guardrail. A determination regarding drainage (need for culverts, size of a culvert, and assessment of possible hazard) will be a controlling factor in deciding geometrics of the bridge for the site.

## 2) Economics

Prestressed concrete beam spans are normally the most economical type of construction for grade separations. In addition, there will usually be greater economy in constructing grade separations using two long spans rather than constructing four shorter spans.

The span lengths and overall bridge length affect the abutment heights, which in turn affect the overall cost of the bridge. See BDM Article 2.3.2 for discussion of bridge types and their economical/typical span ranges as well as a discussion on abutment and pier type selection.

## 3) Appearance

The use of longer spans will necessitate a deeper superstructure and higher approach fills. Consideration should be given to the effect of the depth of structure on the overall appearance and design of the undercrossing.

For rough calculations during preliminary planning, the depth of highway bridge superstructures can be assumed to be about 1/20 of the length of the longest span. (Depth of superstructure refers to the dimension from top of slab to bottom of beam.) Contact the Preliminary Bridge Plans Engineer for the exact dimensions to be used in final planning and for depth of structure on railroad bridges.

### **Lateral Clearance for Roadways**

As noted above, acceptance of the Performance-Based Practical Design (PBPD) philosophy has led to revised lateral clearance requirements at bridge undercrossings. Refer to Technical Memorandum No. 21-05-TS-04 when determining the width requirements for the under-bridge typical section. This technical memorandum can be found at:

<https://techmemos.dot.state.mn.us/techmemo.aspx>

Eliminate side piers wherever possible.

### **Lateral Clearance for Railroads**

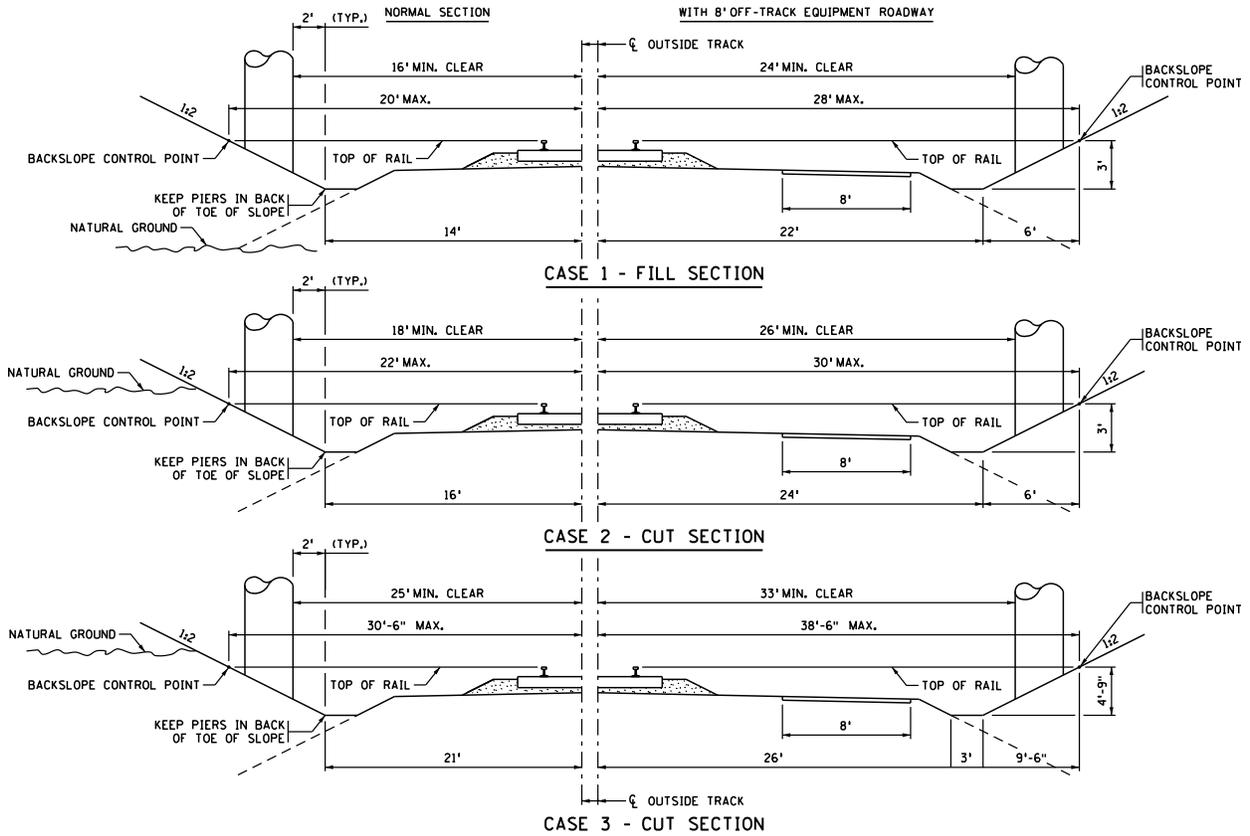
Lateral clearances at railroads are to be determined as follows:

- 1) The statutory clearances diagram shown on Figure 2.1.3.1 represents the absolute minimums that must be adhered to. For design, use a

minimum horizontal clearance of 9'-0" to a pier or abutment (8'-6" is the legal minimum).

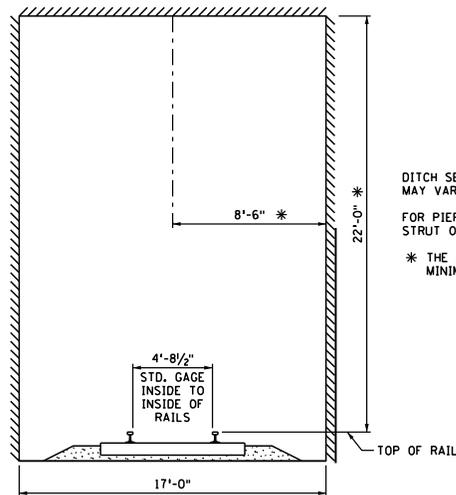
- 2) MnDOT and FHWA historically agreed to the horizontal clearances shown in Figure 2.1.3.1 (25'-0" minimum clearance to pier, 30'-6" to "back slope control point") for mainline BNSF RR tracks at sites meeting the following conditions:
  - a) When the standard will not increase the cost of the structure by more than \$50,000.
  - b) When sufficient vertical clearance exists between the tracks and in-place or proposed roadway profile to accommodate the structure depth necessary for the longer spans typically required by the standard.
- 3) Back slopes shall be 1V:2H and pass through the "back slope control point" shown on Figure 2.1.3.1 for the applicable case. The dimension to the "back slope control point" indicates the maximum extent of federal participation in the construction and must not be exceeded.
- 4) The Preliminary Bridge Plans Engineer will contact the MnDOT Office of Freight and Commercial Vehicle Operations (OFCVO), to negotiate with the railroad the need for provisions for a maintenance road for track maintenance equipment and future track requirements.

The publication *UPRR/BNSF Railway Guidelines for Railroad Grade Separation Projects* includes railway owner design expectations for roadway bridges over railroads, as well as bridges carrying railroads. While this document may include guidance that is unattainable by the MnDOT project at hand, the OVCO has directed the Preliminary Bridge Plans Unit to use this document during plan development and negotiations with the railroads.



**HISTORICAL AGREEMENT BETWEEN MNDOT & FHWA  
 LOCATION OF PIERS AND BACKSLOPES**

BURLINGTON NORTHERN MAINLINE SECTION  
 SEE LATERAL CLEARANCE FOR RAILROADS IN ARTICLE 2.1.3 FOR USAGE



DITCH SECTIONS WITHIN BACKSLOPE CONTROL POINTS MAY VARY WITH RAILROADS.  
 FOR PIERS WITHIN 25' OF TRACK CENTERLINES A COLLISION STRUT OR SOLID SHAFT PIER IS REQUIRED.  
 \* THE 22'-0" AND 8'-6" DIMENSIONS ARE STATUTORY MINIMUM. FOR DESIGN USE 23' AND 9' MINIMUM.

**DIAGRAM OF STATUTORY CLEARANCE  
 FOR STRUCTURES, BRIDGES AND TUNNELS**  
 RAILROAD CLEARANCES

**Figure 2.1.3.1  
 General Railroad Clearances**  
 (Note that railroad approval is required for specific project clearances)

**Waterway Sections Under Bridges**

The Waterway Analysis (hydraulics report) gives information on the required stream cross section under the bridge including waterway area and low member elevation. Potential flood damage, both upstream and downstream, and permitting agencies' requirements must be considered.

The Preliminary Bridge Plans Engineer will coordinate with the Waterway Unit to determine when a wildlife passage bench is required under bridges over waterways.

For bridges on the local system, go to the State Aid Bridge Web Site at <http://www.dot.state.mn.us/stateaid/bridge/resources.html> and refer to the guidance found under Hydraulics.

**Vertical Clearance for Underpasses**

Vertical clearance requirements are aligned with the 2018 edition of the AASHTO *A Policy on Geometric Design of Highways and Streets* (AASHTO GDHS). For highway bridge structures, AASHTO GDHS Article 10.8.4.2 recommends a minimum vertical clearance of 1'-0" above the legal vehicle height, plus an allowance for future pavement resurfacing and other considerations. The legal height of a truck in Minnesota is 13'-6". For pedestrian bridges and sign bridges, AASHTO GDHS Article 5.2.2.7.2 recommends a minimum vertical clearance 1'-0" greater than the highway bridge structure clearance. Table 2.1.3.1 lists the minimum vertical clearance requirements for Minnesota trunk highway underpasses, pedestrian bridges, sign bridges, railroad underpasses, and truss/arch portals. The clearance over highways applies to the traffic lanes and full usable width of shoulders.

Per Minnesota Rules, Chapter 8820, Local State-Aid Route Standards, the minimum vertical clearance for highway underpasses (including construction tolerance) is 16'-4" for rural-suburban designs and 14'-6" for urban designs. For trunk highways crossing local roads or streets at a freeway interchange, the minimum vertical clearance with construction tolerance is 16'-4". A complete list of vertical clearances for local roads and streets is found in the *State-Aid Operations Rules*, Chapter 8820. Note that local road system bridges at an interchange with a trunk highway must be designed to trunk highway standards between the ramp terminals on the local road.

Where bikeways pass under a bridge or through a tunnel, a 10'-0" vertical clearance is desirable for adequate vertical shy distance. (See AASHTO *Guide for the Development of Bicycle Facilities*, 4<sup>th</sup> Edition, pages 5-6 and 5-26.) Where this is impractical, a lesser clearance down to a minimum of

8'-0" is acceptable. Clearances below 10'-0" on the local road system will require a variance to the *State-Aid Operations Rules*, Chapter 8.

**Table 2.1.3.1 Vertical Clearance Requirements for Bridges**

TYPE OF STRUCTURE	MINIMUM VERTICAL CLEARANCE FOR NEW BRIDGES ①②	MINIMUM VERTICAL CLEARANCE UNDER EXISTING BRIDGES FOR ROADWAY PAVEMENT PROJECTS ③
Trunk Highway Under a Roadway or Railroad Bridge on a Super Load OSOW Corridor ④	16'-6"	16'-6"
Trunk Highway Under a Roadway or Railroad Bridge on a Non-Super Load OSOW Corridor	16'-4"	16'-0"
Trunk Highway Under a Pedestrian Bridge on a Super Load OSOW Corridor ⑤	17'-6"	17'-6"
Trunk Highway Under a Pedestrian Bridge on a Non-Super Load OSOW Corridor ⑤	17'-4"	17'-0"
Trunk Highway Under a Sign Bridge on all corridors ⑥	17'-6"	17'-0"
Railroad Under a Trunk Highway Bridge ⑦	23'-0"	NA
Portal Clearance on a Trunk Highway Bridge Through-Truss or Through-Arch ⑧	20'-4"	20'-0"
Portal Clearance on a Railroad Bridge Through-Truss Over a Trunk Highway ⑧	21'-4"	21'-0"

① Provide additional clearance for future resurfacing where practical. Traditional bituminous overlay allowances range from 3" to 6". Un-bonded concrete overlay are greater than the 6" tolerance and can be as high as 12". The appropriate design value will depend on the pavement type, its initial structure type, and lifecycle strategy, and should be coordinated with the Pavement Design Engineer.

- ② A clearance height that includes a future resurfacing allowance may be used in place of the listed minimums, provided the resulting clearance is at least as much as the listed minimums in this column. Construction tolerance requirements for these values have been reviewed and deemed adequate for new bridge construction. Adjust table values upward as required for overlay requirements exceeding 4".
- ③ The minimum vertical clearances shown are the absolute minimum clearances to be achieved after pavement preservation, pavement rehabilitation, or pavement reconstruction under an existing bridge or structure. These minimums are only acceptable due to the known spatial location of the existing structure, thus eliminating the construction tolerance risk of that existing element.

These minimums do not apply to existing bridge repair projects. Refer to MnDOT [Bridge Preservation and Improvement Guidelines](#) for the minimum vertical clearances for bridge preservation and bridge rehabilitation projects.

- ④ A minimum vertical clearance of 16'-6" is required under highway and railroad bridges on designated Super Load OSOW Corridors. Super Load OSOW Corridors are designed to accommodate an envelope size of 16' wide x 16' high x 130' long, traveling along the corridor. For specific locations, the clearance for existing bridges for pavement projects may be reduced with approval from the MnDOT Office of Freight and Commercial Vehicle Operations (OFCVO). Contact the OFCVO for specific corridor locations and requirements: <http://www.dot.state.mn.us/cvo/index.html>
- ⑤ Additional vertical clearance is required under pedestrian bridges because they are much less substantial than highway and railroad bridges and could collapse in the event of a hit. The 1'-0" of additional vertical clearance meets AASHTO GDHS Article 5.2.2.7.2 recommendations.
- ⑥ Additional vertical clearance is required under sign bridges because they are much less substantial than highway and railroad bridges and could collapse in the event of a hit. For new sign bridges on all corridors and for existing sign bridges on non-Super Load OSOW Corridors, the minimum vertical clearances listed meet or exceed AASHTO GDHS Article 5.2.2.7.2 recommendations. The minimum vertical clearance for existing sign bridges on Super Load OSOW Corridors is less than AASHTO GDHS Article 5.2.2.7.2 recommendations. This allows room for pavement mill & overlay projects before a sign bridge must be raised or replaced. Based on consideration of many factors (OSOW 16'-0" height limit, a strict permitting process for OSOW vehicles, cause and frequency of hits, low risk of sign bridge occupancy when hit, cost of replacement, sign structure service life), 17'-0" is deemed adequate for existing bridges.
- ⑦ Vertical clearance over railroad infrastructure requires approval of the railroad. The 23'-0" clearance above top of rails is the minimum clearance required by the American Railway Engineering and Maintenance of Way Association (AREMA) manual. The maximum vertical clearance for Federal Cost Participation is 7.1 meters (23'-4") per the Code of Federal Regulations (see CFR 646 Appendix to Subpart B of Part 646). This is allowed where the railroad's standard practice is to accommodate future ballasting of the tracks. The Minnesota statutory minimum vertical clearance is 22'-0". See Figure 2.1.3.1. For clearances below 22'-0", approval from the MnDOT Office of Freight and Commercial Vehicle Operations (OFCVO) and the railroad is required. Contact the Rail Safety and Coordination Project Manager from the OFCVO for assistance with railroad coordination, agreements, and approvals.

- ⑧ Minimum portal clearance values were set based on historical portal heights. Railroad through-truss bridges require an additional 1'-0" of portal clearance to meet railroad stipulations.

**Vertical Clearance over Waterways**

The location and project description for all bridges over Minnesota waterways are to be reviewed by the U.S. Coast Guard (USCG) for potential permitting requirements.

## 1) Non-Navigable Waterways

A 3'-0" minimum clearance between the 50-year flood stage and low point on the bridge superstructure is recommended. This amount of clearance is desired to provide for larger floods and for the passage of ice and/or debris. If this amount of clearance is not attainable due to constraints relating to structure depth, roadway grades or other factors, reduced clearance may be allowed. The Preliminary Bridge Plans Engineer, after consultation with the Waterway Unit and the MnDOT District Office, will determine the required clearance.

## 2) Navigable Waterways

a) Examples of waterways that require a construction permit (generally considered to be waterways for commercial shipping) from the USCG include:

- The Mississippi River downstream from the railroad bridge that crosses the river south of 42<sup>nd</sup> Avenue North in Minneapolis (River Mile Point 857.6)
- The Minnesota River downstream from location just west of T.H. 101 river crossing in Shakopee (River Mile Point 25.6)
- The St. Croix River downstream from Taylors Falls
- The St. Louis River downstream from Oliver, Wisconsin.

Guide vertical clearances published by the USCG are:

- Mississippi River:
  - 52.0' above 2% flowline elevation or 60.0' above normal pool elevation, whichever is greater, for the portion downstream of the Burlington Northern Railroad Bridge near the University of Minnesota (River Mile Point 853.0).
  - 21.4' above river stage of 40,000 c.f.s. for the river portion upstream (River Mile Point 853.0 to 857.6).
- Minnesota River:
  - 55.0' above normal pool elevation from the river mouth to I-35W bridge (River Mile Point 10.8).
  - 30.8' above 1881 high water elevation from I-35W bridge (River Mile Point 10.8) to Shakopee (River Mile Point 25.6).
- St. Croix River:
  - 52.0' above 2% flowline elevation or 60.0' above normal pool elevation, whichever is greater, from the river mouth to Stillwater.

- Lake Superior Watershed:
  - Navigation clearances are determined by USCG on a case-by-case basis.

Consult the Preliminary Bridge Plans Engineer when establishing navigation clearances.

b) All Other Navigable Waterways

Bridges that cross waterways in other portions of the state may be required to provide for local pleasure boat traffic. Vertical clearance for these bridges will be determined on an individual basis, based on local needs. The Preliminary Bridge Plans Engineer, in consultation/concurrence with the Waterway Unit, the MnDOT District Office, and the MnDNR, validate the design based on specific conditions of the waterway.

### **Vertical and Horizontal Alignment**

Information governing vertical curves, horizontal curves, and sight distance may be found in the *Facilities Design Manual* and *Technical Manual*.

Note that in MnDOT plans the term “azimuth” refers to the angle of horizontal deviation, measured clockwise, from true north to the direction of increasing stationing.

Example: Consider a straight line horizontal alignment that runs exactly east-west. If stationing increases from west to east, azimuth is 90 degrees. If stationing increases from east to west, azimuth is 270 degrees.

When preparing preliminary bridge plans for the local road system, refer to the [State Aid Manual](#) for vertical and horizontal alignment requirements.

#### **2.1.4 Bridge Barriers and Railings**

See Section 13 of this manual for the policy on design of bridge barriers and railings for MnDOT projects.

#### **2.2 Bridge Aesthetics**

The aesthetic design process is initiated early in the bridge planning phase.

The Preliminary Bridge Plans Engineer, the Preliminary Bridge Architectural Specialist, the District, and the financial stakeholders determine the aesthetic design level with an eye on constructability and cost. Other people, offices, agencies, etc. may also be involved. The extent of this involvement may vary depending on the individual project. This process leads to the development of an Aesthetic Plan for the bridge. Once the

project reaches the final stage, the Bridge Design Unit Leader implements the Aesthetic Plan to completion with assistance from the Preliminary Bridge Architectural Specialist as needed.

Note that constructability of aesthetic components and complexity of the aesthetic details may affect the project schedule, and therefore must be considered during the development process.

Section 3 of the [Aesthetic Guidelines for Bridge Design Manual](#) describes the process of aesthetic design in more detail.

Maximum levels of MnDOT participation in aesthetic costs are given in the [Cost Participation and Maintenance Responsibilities with Local Units of Government Manual](#).

## **2.3 Preliminary Bridge Plans**

### **2.3.1 General**

#### **Purpose**

The Preliminary Bridge Plan serves to document the main features of the bridge (type, size, location, aesthetics, etc.) and is used to obtain approvals and coordination before final design begins. By doing this, the time and expense of revising a completed plan will hopefully be avoided. The plan coordinates the work between Road Design and the Bridge Office and enables the cost and scope of the work to be estimated.

Specific users of the plan include:

- Road Designers to verify the grade, alignment and roadway widths and to obtain the approximate limits of grading, paving and guardrail at the bridge ends.
- FHWA to review and approve unusual or complex bridge projects.
- Bridge Office Consultant Agreements Unit to select and negotiate contracts with consultants.
- Final Bridge Design Units and Consultants to prepare final plans.
- Bridge Scoping Engineer and Bridge Estimates Unit to prepare a preliminary estimate of the bridge costs.
- MnDNR, U.S. Coast Guard, U.S. Corps of Engineers, and Watershed Districts to review and issue required permits for stream crossings.
- Cities, Planning Agencies, and citizen groups to review and approve projects.
- District Traffic Engineer and Regional Transportation Management Center (RTMC) to convey their needs on the new bridge.
- MnDOT Office of Freight and Commercial Vehicle Operations (OFCVO) for use in negotiating railroad agreements.

In preparing preliminary bridge plans, the plan users should always be kept in mind, particularly those without bridge technical experience.

### **Requirements for Preliminary Bridge Plans**

Preliminary bridge plans are required for all new trunk highway bridges (including MnDOT precast concrete arch and three-sided structures and pedestrian underpass box culverts) and all bridge widening projects where substructure widening is required. In addition, preliminary plans signed by the State-Aid Bridge Engineer are required for all county and local bridges that cross a trunk highway. Preliminary bridge plans are not required for culverts (except those used for pedestrian access), overlays, deck replacements, and other projects where substructures are not widened.

The Bridge Preliminary Plans Unit normally prepares preliminary plans for new trunk highway bridges, although consultants may also develop plans. Preliminary plans for bridge widening projects are normally prepared by the Bridge Design Units since significant design work is required to evaluate the existing structure and schemes for widening and handling traffic.

Preliminary plans prepared by Consultants or Design Units are submitted to the Bridge Preliminary Unit for review, acceptance, submittal to the State Bridge Engineer for signature, and distribution of signed copies.

### **Contents**

The Preliminary Bridge Plan consists of a general plan and elevation sheet, survey sheet, and borings sheet. For complex urban structures additional road design sheets giving alignment, superelevation diagrams, utilities, contours, traffic staging, intersection layout, and aesthetics may be included. The Preliminary Bridge Plan contains: plan and elevation views, a transverse section, design data, data on the type of structure, foundation requirements, and aesthetic treatment. When aesthetics are of special importance, architectural type drawings showing the proposed treatment or type of construction may also be included. For bridge widening projects, the survey sheet may be eliminated or a copy of the survey sheet from the existing bridge may be included.

### **Preparation of Preliminary Bridge Plans**

The steps involved, although not necessarily in order, in preparing a typical preliminary plan set for a new trunk highway bridge by the Preliminary Unit are as follows:

- 1) *Request for Bridge Scoping and Cost Estimating Assessment – Bridge Replacement* (Form A) is completed, which provides the initial information for the project. Form A is found at:

<http://www.dot.state.mn.us/bridge/scoping.html>

Consideration is given to the use of Accelerated Bridge Construction (ABC) methods at this step in the process. Results of the ABC Stage 1 assessment is reported on Form A and, if applicable, the ABC Stage 2 assessment is completed and included as an attachment. In addition, a bridge number is requested by the MnDOT District Project Manager or their designee by completing the online *New bridge number request form*, found at:

<http://www.dot.state.mn.us/bridge/bridgereports/index.html>

A new bridge number is then assigned.

- 2) Approved geometric layouts are received from the District.
- 3) Bridge survey sheets are received from the District Surveys Section. Copies are sent to the Foundations Unit of the Office of Materials and Road Research requesting soil borings. For stream crossings, a copy is sent to the Bridge Office Waterway Unit requesting a waterway analysis.
- 4) A depth of structure and span arrangement are determined using the approved geometric layout and waterway analysis (if applicable) and are given to Road Design. This typically involves communication between the Bridge Office, Road Design, and Hydraulics to arrive at a structure depth and span arrangement that produces the best overall solution. Note that when calculating structure depth for preliminary plan purposes, assume that the beam stool plus the beam camber equals a depth of 4 inches.
- 5) If a railroad is involved, negotiations are held with the railroad to determine what features should be incorporated into the plan to satisfy the railroad's needs and also meet MnDOT standards.
- 6) Final grades and alignment are developed and officially received from Road Design.
- 7) A CADD technician is assigned the project and drafting of the plan begins. Clearances are checked and more exact span lengths determined.
- 8) Traffic data is requested and received from the District Traffic Engineer.
- 9) The extent of aesthetic treatment is determined following the process described in BDM Article 2.2.
- 10) Deep foundation borings are received electronically from the Foundations Unit along with the *Foundation Analysis and Design Recommendations* (FADR). The borings are plotted on the survey sheets.
- 11) The Preliminary Bridge Plans Unit checks the completed preliminary package, except for finalizing the foundation type.
- 12) The preliminary package is given to the Regional Bridge Construction Engineer along with the FADR for determining foundation type; this

includes pile type, lengths, and resistances. When received, the final foundation information is added to the preliminary plan.

- 13) The completed Preliminary Bridge Plan is reviewed with the Bridge Planning and Hydraulics Engineer and taken to the State Bridge Engineer for signature.

**Time Schedule for Preliminary Plan Preparation**

The time schedule for receiving information and completing preliminary bridge plans for normal bridges, as given in Primavera P6, is shown in Table 2.3.1.1.

**Table 2.3.1.1 Preliminary Plan Time Schedule**

WORK ITEM	TIME PRIOR TO SCHEDULED LETTING DATE
Bridge Survey	21 months
Hydraulics	18 ½ months
Grades and Alignment	18 ½ months
Foundations	17 months
Preliminary Plan Completed	16 months (typical bridges) 20 months (major bridges)

Additional lead-time beyond that given in the table above is required for major bridges, bridges involving agreements with cities or railroads, and bridges with extensive aesthetic requirements.

In addition to the work items listed above, time must be allotted for a formal type selection study for major bridges.

**Use of Preliminary Bridge Plans**

The completed and signed Preliminary Bridge Plan becomes the department’s official proposal for that structure. The following steps are then taken:

- 1) The Bridge Estimating Unit in the Bridge Office prepares an estimated contract construction cost for the structure.
- 2) Copies of the Preliminary Bridge Plan are distributed to the various offices of MnDOT and outside agencies for information, review, and approval, as the case may be. (See Table 2.3.1.2.)

Approval by all concerned of the proposed structure dimensions, type of construction, and geometrics before the start of final design is one

of the most important functions of the Preliminary Bridge Plan. This is particularly true of stream crossings, railroad crossings (over and under), and structures requiring special aesthetic treatment.

The Federal-Aid Highway Program (FAHP) provides federal-aid to State-selected projects. The Federal Highway Administration (FHWA) administers the FAHP on behalf of the U.S. Secretary of Transportation under Title 23 and therefore is one of the outside agencies that reviews bridge projects. The *FHWA Minnesota Division and Minnesota Department of Transportation Stewardship & Oversight Agreement* documents the roles and responsibilities of the FHWA and MnDOT regarding project approvals and review:

- For most bridge projects, MnDOT assumes the FHWA's Title 23 responsibilities and only a courtesy copy of the Preliminary Bridge Plan transmittal letter is sent to FHWA (without the plans) for informational purposes.
- For unusual or complex bridges and structures, the FHWA Minnesota Division is responsible for the approval of the Preliminary Bridge Plan. For the purpose of this guidance, unusual or complex bridges and structures are defined as those that the FHWA Minnesota Division determines to have unique foundation problems, new or complex designs, exceptionally long spans, exceptionally large foundations, complex hydrologic aspects, complex hydraulic elements or scour related elements, or that are designed with procedures that depart from currently recognized acceptable practices. Examples of unusual or complex bridges and structures include cable-stayed bridges, suspension bridges, arch bridges, segmental concrete bridges, movable bridges, truss bridges, tunnels, complex geotechnical wall systems, and complex ground improvement systems.

When submitting preliminary documents to the FHWA, include the Preliminary Bridge Plan and supporting information. Supporting information includes all bridge/structures related environmental concerns and suggested mitigation measures, studies of bridge types and span arrangements, approach bridge span layout plans and profile sheets, controlling vertical and horizontal clearance requirements, roadway geometry, design specifications used, special design criteria, special provisions (if available), and cost estimates. In addition, submit hydraulic and scour design studies/reports which show scour predictions and related mitigation

measures. Also submit geotechnical studies/reports along with information on substructure and foundation types.

For unusual or complex bridge projects, the State Bridge Engineer will submit one copy of the Preliminary Bridge Plan along with a transmittal letter requesting approval directly to the FHWA Division Bridge Engineer. The transmittal letter also includes the estimated contract construction cost of the structure. The FHWA is the only outside agency to which the Bridge Office sends a direct request for approval. All other outside agencies are contacted through other offices of MnDOT.

Note that the FHWA Headquarters Bridge Division is available for technical assistance on other Federal-aid and non-Federal-aid highways when requested.

- 3) The Preliminary Bridge Plan is used as a basis for preparing permit drawings to accompany applications to construct structures and approaches over navigable waters of the United States within or bordering our state. Such drawings are prepared in the Preliminary Plans Unit in accordance with detailed instructions issued by the U.S. Coast Guard. The Coast Guard is charged with the responsibility of issuing permits for bridges over navigable waters of the United States within or bordering our state. This includes all bridge spans (including land spans) from abutment to abutment. The Corps of Engineers is responsible for issuing permits for any other miscellaneous structures or work to be performed in navigable waters of the United States.

There are two Coast Guard districts that have jurisdiction within the State of Minnesota; the 9<sup>th</sup> Coast Guard District based in Cleveland has jurisdiction over the Duluth harbor and navigable portion of the St. Louis River, and the 8<sup>th</sup> Coast Guard District based in St. Louis has jurisdiction over the navigable portions of the Mississippi, Minnesota, and St. Croix Rivers.

After receiving a permit application, the Coast Guard issues a public notice of application with prints of the permit drawings. These are sent to shipping interests, other agencies, displayed in post offices, etc. Generally, if no comments are received from others within 30 days of the notice of application, and if environmental statements have been submitted and a certification given by the Minnesota Pollution Control Agency, a permit will be issued.

Correspondence to the Coast Guard is generally prepared for the signature of the State Bridge Engineer.

- 4) When all approvals have been obtained, the Preliminary Bridge Plan is used as the basis for the bridge design and for the preparation of final detailed plans. If the design is to be by a consulting engineer, the Preliminary Bridge Plan is typically used as the basis for negotiation of the consultant fee.

**Table 2.3.1.2 General Distribution of Preliminary Bridge Plans**

DISTRIBUTION TO	PURPOSE		REMARKS
	INFO. & REVIEW	PRE-APPROVAL REQUIRED	
MnDOT District Project Manager	x		
District Pre-Design	x		
District Final Design	x		
District Construction	x		
District Environmental Coordinator	x		
District Hydraulics Engineer	x		For bridges that cross waterways.
District Maintenance	x		
District Bridge Engineer	x		
District Traffic Engineer	x		Send with request for determination of need for lights, signals, conduit, and bridge mounted signs.
Office of Materials & Road Research – Foundations Unit	x		
Regional Transportation Management Center	x		Send with request for determination of need for conduit and mounting devices for surveillance system.
Environmental Stewardship Office	x		
MnDOT Office of Freight and Commercial Vehicle Operations (OFCVO)		x	For railroad crossings only.
Federal Highway Administration (FHWA)	x	x	Approval required for unusual or complex bridge projects only. For all other bridges, a courtesy copy is provided.
Bridge Final Design Unit	x		
Bridge Estimating Unit	x		
Bridge Waterway Unit	x		For bridges that cross waterways.
Bridge Consultant Agreements Unit	x		For bridge projects with consultant involvement.
Bridge Consultant	x		For bridge projects with consultant involvement.
Other Stakeholders	x		As needed.

**Preliminary Plans for Local Bridges**

Consult the State Aid Bridge Web site at:

<http://www.dot.state.mn.us/stateaid/bridge/resources.html> for the submittal and acceptance process of State Aid Preliminary Bridge Plans.

**2.3.2 Bridge Type Selection****General**

The type of structure and span arrangement selected will depend on cost, depth available, geometrics, site conditions, and aesthetics. For some bridges this may be an obvious choice. For others it may involve a great deal of study, especially if aesthetics is a main concern. The section that follows gives some general guidelines on the selection process.

**Aesthetic Design Process**

See Section 2.2 of this manual for a general discussion of the aesthetic design process.

**Structure Type**

The most commonly used structure types and their characteristics are as follows:

**1) Precast Pretensioned Concrete Beam**

This is the most common structure type in Minnesota. Advantages include: low initial and future maintenance costs, high quality factory produced product, a stiff deck, and simple spans that accommodate tapers. Beams are limited to standard depths and straight segments, and a maximum length of about 200 feet. Beams in excess of 150 feet may require special shipping considerations.

**2) Welded or Rolled Steel Beam**

This type of structure is well suited to complex urban freeways with limited depth, long spans, and complex geometrics. Steel beam bridges are also well suited for areas with bad soils, such as the Red River Valley, as steel allows the flexibility of modifying the bearing location and adding or reducing span lengths to accommodate shifting abutments and piers. Advantages include: a shallower depth of structure than prestressed concrete, beams with the ability to be field spliced to produce long span lengths, web plates that can be cut to any depth or to a haunched shape, beams that can be curved horizontally, and beams that can be painted a color which contrasts with the slab to make the structure appear thinner. Disadvantages include: a typically higher cost than other structure types, more difficult fabrication and inspection, a longer fabrication time, the possible need for initial painting and future maintenance painting, weathering steel staining of supports, and rusting of weathering steel when under salt exposure.

3) Cast-In-Place Concrete Slab Span

This type of structure is used for shorter span bridges where depth is a major consideration. For simple spans conventionally reinforced, spans range up to 40 feet. Continuous spans are limited to about 60 feet. (See table in Section 5.3.1 of this manual for limits.) Advantages include: a minimum depth superstructure, ease of design and detailing, pleasing aesthetics, and economy for short span bridges. Disadvantages include: span lengths are limited, falsework is required, concrete delivery rate requirements may be a problem, a wearing course may be required to achieve a smooth ride, and the maximum skew angle is 45°.

4) Post-tensioned Concrete Box Girder

Concrete box girders provide an attractive structure with high torsional resistance making them especially well suited for curved structures. The ability to accommodate an integral pier cap is an advantage since horizontal clearance is only required to the column top and not the cap top. Limitations and drawbacks may include the need for falsework, the inability to redeck or widen, and the higher construction cost.

5) Timber

This bridge structure is used only on the local road system, for 1 or 3 spans with a maximum span length of about 25 feet. Advantages include: timber has a natural and aesthetically pleasing appearance, special equipment is not required for installation, and construction can be done in virtually any weather conditions. Disadvantages include: timber is not an economical structure type, it is limited to low-volume roads (roads with an AADT under 750), and the asphalt wearing surface tends to crack due to differential deck deflections.

6) Precast Concrete Box Culvert

Box culverts provide a quickly constructed and economical structure for stream crossings and pedestrian tunnels. Precast concrete box culvert standards are available for culverts up to 16 ft. x 12 ft. in size. Use of up to three large barrel boxes may be economical compared with a bridge. Advantages include: standardized plans, quick installation and low maintenance. Disadvantages include: span limitations, possible debris build-up when multiple barrels are used, and lack of a natural stream for fish unless the invert is lowered and riprapped.

7) Three-Sided Bridge Structure

Three-sided precast concrete structures offer an alternative for short span structures up to 42 feet. Advantages include: quick installation, and a natural stream bottom if scour protection is not required.

Disadvantages include: a higher cost than cast-in-place structures, and pile foundations are typically required for stream crossings unless founded on rock.

Not all bridge sites lend themselves to the use of the more common bridge types listed above. For these situations, specialized bridge types may be required, such as post-tensioned I-girder bridges, tied arch bridges, cable-stayed bridges, or extradosed bridges.

### **Abutment and Pier Locations**

The following guidelines aid in setting abutment and pier locations:

#### 1) Water Crossings

For water crossings, keep the number of substructures located in the water to the minimum practical. Piers in rivers and streams block the natural flow of the waterway, trap ice and debris, impede navigation, and are subject to scour. In addition, construction of a pier in the water is expensive (especially if cofferdams are needed), and environmentally disturbs the stream/river/lake bottom and water quality. Ideally, set piers and abutments on shore to minimize dewatering and allow easy access for the Contractor. Set substructures to avoid interference with in-place substructures, including piling, wherever practical. Setting spans and structure depth involves balancing the hydraulic requirements of the low member elevation and waterway area with the constraints of approach grades, structure depth, and cost.

#### 2) Grade Separations

For grade separations, fewer piers are also desirable wherever practical. When determining pier locations adjacent to roadways, refer to Technical Memorandum No. 21-05-TS-04, which can be found at:

<https://techmemos.dot.state.mn.us/techmemo.aspx>

In locations where ramps enter or exit a highway under a bridge, avoid piers between the mainline and ramp, if possible, as they restrict visibility.

Piers to be placed in the median must be located with consideration of the project parameters, including: safety, current and future roadways, available buffer distance between traffic and piers in both directions, cost, etc. For a typical divided highway median pier, place the pier equidistant to current lanes in both directions. Where location of future lanes is markedly different from the current configuration, discuss placement of median pier with the District and the Preliminary Bridge Plans Engineer to determine the best pier location for the current and future configurations.

**Abutment Types**

Abutments can generally be classified into 3 categories: stub, semi-high, and high abutments. A further breakdown of abutments can be made according to the way expansion is handled – integral, semi-integral, or parapet type.

- 1) **Stub Abutment:** This is the shortest category of abutment, located at the top of the fill slope with generally 2 to 4 feet of stem exposure.

Integral type stub abutments are the preferred type of abutment due to their jointless nature and simplified construction. Integral type stub abutments have the lowest initial construction cost, are the fastest abutment type to construct, and eliminate the future maintenance and repair required for strip seal expansion joints. Refer to BDM Article 11.1 for length, skew, and exposure limits for integral type stub abutments.

Semi-integral type stub abutments are the preferred type of abutment when the requirements for integral abutments cannot be met. Semi-integral abutments have a lower initial construction cost than parapet type abutments, and eliminate the future maintenance and repair required for strip seal expansion joints. Refer to BDM Article 11.1 for length and skew limits for semi-integral type abutments.

Parapet type stub abutments use a strip seal or modular expansion device to accommodate movement. They have the highest initial construction cost, and will require future maintenance and repair for the strip seal expansion joints. The move toward jointless abutments has diminished the use of parapet type stub abutments, but this type is still used where appropriate.

- 2) **Semi-high abutment:** This abutment type is located part way up the fill slope and became more popular as two-span overpasses came into use. A slightly higher abutment and elimination of the berm reduces the span length and depth of beam. This allows a lesser profile grade increase, resulting in lower grading costs. Limit the exposed height of abutment face to approximately 8 feet, if possible. Undertake a cost evaluation of longer spans vs. taller abutments when considering a semi-high abutment.

This category includes semi-integral and parapet type abutments only (integral abutment height restrictions limit them to the stub abutment category). Semi-integral type abutments are the preferred type

because of their lower initial construction cost and lower maintenance requirements.

- 3) High abutment: This abutment type is located at the bottom of the fill slope and is used primarily in congested urban design where structure depth is difficult to obtain. Their use is discouraged since they are more difficult to construct, expensive, require lengthy retaining walls and approach panels, and give a closed-in feel to the highway. Again, this category includes semi-integral and parapet type abutments only, with the semi-integral type preferred due to its lower initial construction cost and lower maintenance requirements.

In locations where a high abutment would be required and use of a mechanically stabilized earth (MSE) retaining wall is economical, another option is a parapet type abutment supported by a pile foundation behind an MSE retaining wall.

Things to consider when deciding on what height of abutment will best serve a specific project include:

- Advantages of choosing a shorter abutment over a taller abutment:
  - Lower abutment cost.
  - Longer bridge length results in reduced grading and pavement cost.
  - Shorter wingwall and approach panel lengths.
  - Construction of abutments farther from roadway underneath allows for construction staging, possible future expansion underneath for roadway widening or addition of sidewalks, shared-use trails, or other facilities.
- Disadvantages of choosing a shorter abutment over a taller abutment:
  - Requires a longer bridge length, resulting in a higher superstructure cost and increased future maintenance cost.
  - May require a grade raise, resulting in a higher roadway grading and pavement cost.
  - Higher cost for slope protection.

Wingwalls parallel to the bridge roadway are used most often for aesthetic reasons. Flared wingwalls, typically with a flare angle of 45 degrees for bridges with no skew, will result in shorter wingwall lengths and less length of railing. Straight wingwalls, an extension of the abutment parapet, are the simplest to construct but are appropriate only for shallow beams where aesthetics is not a concern.

See additional limitations and guidance for integral, semi-integral, and parapet abutments in Section 11 of this manual.

### **Pier Types**

There is a wide variety of pier types used in bridges, with the most typical consisting of a pier cap supported on multiple columns.

Consider the following general guidelines in order to achieve cost effective piers:

- Minimize the number and size of the columns.
- Minimize the pier width.
- Minimize the number of construction joints in the columns and the pier cap.
- For ease of reinforcement detailing, avoid use of inside corners in the pier column/shaft cross-section.
- For column and cap type piers that require a crash strut, consider use of a solid shaft type pier to reduce construction time and cost.

The feature being crossed is an important consideration when choosing the pier type. Discussions on pier type are provided below for water crossings and grade separations.

#### 1) Water Crossings

**Pile Bent Piers:** These piers consist of a row of piles with a concrete cap encasing the pile top, and are the simplest and most economical type of pier. They are used for water crossings where a general maximum height from the top of pier to stream/river/lake bed is under 20'-0" and there is no ice or debris problem. Note that it is important to confirm by analysis that the pile unbraced length under a scour condition does not create instability in the pile. Spans must also be short enough to allow a single row of piles to support the deck at reasonable spacing. The piles act as columns, and bending strength to resist side impacts from ice or debris is important. For cast-in-place piles (the most widely used), a 16" minimum diameter is required. If H-piles are used, the upper portion is encased by a cast-in-place pile shell filled with concrete. Timber piles are not permitted. Concerns with pile bent piers include the potential to trap debris, pile stability, and appearance.

**Wall Type Piers:** These piers consist of a single row of piles (usually H-piles) encased with concrete to form a wall. They provide more resistance to ice and debris and allow debris to pass through without becoming entangled on the piles. This type of pier is used where more resistance to ice and debris than afforded by the pile bent is needed, and yet the size and expense of a solid shaft pier can be avoided. This

type of pier can be constructed by driving the piling, supporting the wall forms on the stream/river/lake bed, placing a seal with a tremie, dewatering, adding reinforcement, and pouring the wall. Pile stability can be a concern and must be evaluated.

**Solid Shaft or Multiple Column Piers:** These piers are used for major water crossings where tall piers are required or where heavy loads or sizable ice and debris loads may occur. This type of pier includes a footing with the bottom of footing located a minimum of 6'-0" below the stream/river/lake bed. Construction of this type of pier involves driving sheeting to form a cofferdam, excavating inside the cofferdam, driving piles, pouring a seal, dewatering, and placing concrete.

## 2) Grade Separations

Piers at grade separations are typically multiple column type with a cap. Piers are visible to passing motorists and the emphasis on aesthetics has led to more use of rectangular shaped column type piers, often with form liner treatments or rustication grooves. For narrow ramp bridges, a single shaft pier may be considered. Where aesthetics is not a concern, a round column pier will usually provide the lowest cost.

For the majority of bridges over roadways, piers located within 30 feet of the roadway edge (defined as the edge of the lane nearest to the pier) must be designed to withstand a 600 kip load unless they are protected as specified in LRFD 3.6.5.1. This may impact the aesthetics by requiring inclusion of a crash strut. The alternative is to provide columns with a substantial cross-section designed to resist the crash load or protect them with a TL-5 barrier. See Article 11.2.3 of this manual for complete pier protection policy and requirements.

For bridges over railroads, piers located within 25 feet of the centerline of railroad tracks must either be of "heavy construction" or have crash walls. Refer to Article 11.2.3.2.2 of this manual for complete requirements.

See Section 11 of this manual for additional guidance on piers.

### **2.4 Final Bridge Plans and Special Provisions**

The primary purpose for preparing the Final Bridge Plan and special provisions is to communicate the geometric, material, and procedural requirements for the construction of a bridge. Several audiences will use the final bridge plan or contract documents during the life of the bridge. Initially, contractors use the documents to prepare their bids. A clear, accurate, and complete set of documents will result in competitive bidding.

Well-communicated information reduces contractor uncertainty regarding what is required for different elements of construction.

During construction, many parties will use the contract documents. For example, surveyors will locate and mark the position of working points, fabricators and construction engineers will prepare shop drawings and other submittals/drawings, inspectors and suppliers will use the documents for their work, and the contractor's forces will use the documents.

After construction of the bridge the detailed plans will be referenced when modifying the bridge (e.g., adding signage), performing load rating of the bridge, or rehabilitating/replacing the bridge.

The Final Bridge Plan contains geometric information, a schedule of quantities and pay items for the bridge, traffic phasing (if applicable), limits of removal of existing structures and foundation items (if applicable), foundation details, substructure details, superstructure details, typical sections, utilities (if applicable), survey information, and other miscellaneous items.

Specifications are also required for each project. They describe procedures for award and execution of the contract, how work will be measured and paid, procedures to be followed during execution of the work, and material and testing requirements for items incorporated into the project.

Bridge projects use specifications from four different sources:

- 1) Most of the specifications used for a project are provided in MnDOT's *Standard Specifications for Construction*. They are necessarily general in nature and are intended to cover all types of MnDOT projects.
- 2) The Bridge Office has assembled additional specifications. Because they are not included in the standard specifications they are called special provisions. A list of available standard bridge special provisions (*2018 "SB" Bridge Special Provisions*) is provided on the Bridge Office web site at: <http://www.dot.state.mn.us/bridge/construction.html>. Special provisions address a variety of work items, ranging from concrete placement to the fabrication and installation of expansion joint devices. Not all of the special provisions are intended to be used on every project; use only those applicable to the project.
- 3) The Bridge State Aid Unit has additional standard bridge special provisions that apply to local road bridge projects.

- 4) Custom special provisions. If a work item is of such unique character that the standard specifications and the standard bridge special provisions don't describe or address the work, a custom special provision will need to be prepared. Custom special provisions may be generated for any number of items. Items may include schedules (e.g., dates the contractor will have access to certain portions of the project) or lists of required submittals, etc.

In general, information that is highly graphical or geometric in nature should be presented on plan sheets. Large amounts of information conveyed with text should be assembled in special provisions.

A specification or special provision usually contains the following five sections:

- 1) Description of work
- 2) List of the materials used (and their specifications)
- 3) Construction requirements for the work
- 4) Description of how the work will be measured
- 5) Basis of payment (pay item for the work)

#### **2.4.1 Final Design Instructions**

Unless specified otherwise within this manual, design all structures in accordance with the current *AASHTO LRFD Bridge Design Specifications*. For those few cases where LRFD specifications have not been created or adopted, discuss options with the State Bridge Design Engineer prior to beginning final design.

Design railroad bridges according to the current AREMA specifications for the live load specified by the railroad. Additional notes concerning the design of railroad bridges:

- 1) Railroad bridges will usually be designed with simple spans to avoid uplift from the live load.
- 2) Bridges for the Duluth Mesabe & Iron Range Railway require a special live load.

Plans and documents prepared during the preliminary design phase should be reviewed prior to beginning final design. These documents include:

- 1) Preliminary Bridge Plan
- 2) Bridge Construction Unit Foundation Recommendation Report
- 3) Design Study Report (if completed)
- 4) Preliminary Design Folder (found in ProjectWise)

When reviewing preliminary plans, pay particular attention to geometry and utilities. Check the layout. This includes reviewing grades, stationing, end slopes, beams, railings, roadways, shoulders, and the median (if applicable).

#### **2.4.1.1 Superstructure**

Space beams so moments in fascia beams will not be larger than moments in interior beams.

##### **2.4.1.1.1 Framing Plan**

For steel beams and pretensioned I-beams, deck projections beyond the centerline of the fascia beam should generally not exceed the smallest of:

- 1) Depth of beam: During construction, overhang support brackets that support deck forms, safety walkway, etc., contain a diagonal member that is supported off the beam bottom flange. If the overhang exceeds the beam depth, analyze to check if bracing of the beam is required. Include a note in the bridge plan if bracing is required.
- 2) 40% of the beam spacing: This limit keeps the deck overhang moment and the exterior beam dead load within a reasonable range. If exceeded, Section 9 deck tables cannot be used and a special design is required.
- 3) Deck coping width + barrier width + 1'-0" + ½ flange width: This keeps the design truck wheel within the limits of the exterior beam top flange, thereby ensuring that the live load will not govern the deck overhang design.

For rectangular pretensioned beams, deck projections beyond the centerline of the fascia beam should generally not exceed the smaller of:

- 1) 40% of the beam spacing: This limit keeps the deck overhang moment and the exterior beam dead load within a reasonable range. If exceeded, Section 9 deck tables cannot be used and a special design is required.
- 2) Deck coping width + barrier width + 1'-0" + ½ beam width: This keeps the design truck wheel within the limits of the exterior beam top flange, thereby ensuring that the live load will not govern the deck overhang design.

Provide a minimum slab projection beyond the tip of the flange of 6 inches.

##### **2.4.1.1.2 Bridge Decks and Slabs**

For bridges with reinforced concrete decks or slabs, the deck or slab may be cast in one lift (monolithic) or two lifts (deck/slab plus low slump wearing course). Note that the wearing course and the future wearing course are separate and distinct items.

**Bridge Deck Protection Policy**

For new bridge decks and slab span superstructures, utilize:

- High Performance Concrete (3YHPC). In remote areas of the state where ready mix suppliers cannot produce 3YHPC, use Low Cracking High Performance Concrete (3YLCHPC).
- Monolithic Deck or Slab (no separate wearing course).
- Synthetic Fibers (a combination of micro and macro synthetic fibers).

A list of common conditions that may warrant exceptions to the above is provided in Table 2.4.1.1.2.1. Note that these exceptions apply only to bridges with AADT greater than 2,000.

For new bridges in remote areas and bridges that meet any of the conditions found in Table 2.4.1.1.2.1, the Preliminary Bridge Plans Engineer will consult with the Regional Bridge Construction Engineer and the District to determine the appropriate concrete mix and whether a wearing course is required. The concrete mix type and type of deck or slab (monolithic deck/slab or deck/slab with wearing course) will be included in the Preliminary Bridge Plan.

**Table 2.4.1.1.2.1**

**Bridge Conditions That Require Consideration of a Low Slump Wearing Course ①**

Condition	Commentary
1) Project locations where HPC concrete is not available.	Not all MN concrete plants have successful history with HPC concrete production and delivery.
2) Bridge is located on a constant grade < 0.83%.	Variations in superstructure deflections and finishing tolerances can make positive drainage difficult.
3) Bridge has a continuous steel superstructure with degree of curvature > 10 degrees.	Behavior of steel superstructure deflections and rotation during sequential pouring can be difficult to adequately predict to achieve ride tolerance.
4) Skew > 30 degrees on 2 spans or more with an aspect ratio (deck width/span length) > 0.5.	Finishing rails must deflect uniformly to produce the most uniform cross-section. Finishing of skewed bridges is best accomplished by placing wet concrete uniformly on all beams within the span by setting the finishing machine on a similar skew to substructures.
5) Bridge is located on a vertical curve with approach grades > 3% and support skews > 20 degrees.	Where a vertical profile and skew exists, the difference in elevation at either rail may produce a warped superelevation if finished on skew.
6) Superelevation transition occurs on the bridge.	Finishing machines cannot easily accommodate variable superelevation breaks during a pour.
7) Bridge deck or slab has a longitudinal construction joint due to traffic staging or large deck width.	Multiple pour placements with longitudinal construction joints are more prone to cracking during deflections incurred during the casting sequence. A concrete wearing course placed after major deflections have occurred results in better crack size control.
8) Variable width bridges such as single-point interchanges that are difficult to finish with a finishing machine.	Finishing machines have limited ability to expand width, and finishing outside of the screed rail locations requires finishing equipment with higher risk of placement irregularities.

① Applies only to bridges with AADT greater than 2,000.

**2.4.1.1.3  
Diaphragms and  
Cross Frames**

For most bridges, the orientation of the primary superstructure elements is parallel to the centerline of the bridge. Aside from slab bridges, most bridges in Minnesota are supported on multiple beam lines. The beam lines are typically spaced on 5 to 15 foot centers. These bridges usually have diaphragms or cross frames, which serve a number of purposes:

- 1) They provide compression flange bracing during erection and construction of the bridge.

- 2) They increase lateral load distribution (more beams or girders participate in carrying live loads).
- 3) They provide a load path for lateral loads to be carried from the deck to the bearings.

During final plan assembly, specify the type of diaphragm on the framing plan, the deck cross section, and the longitudinal section.

For bridges with integral or semi-integral abutments, the end diaphragm also functions as an abutment element.

#### **2.4.1.2 Pedestrian Bridges**

Pedestrian bridges shall be designed in accordance with the *Guide Specifications for Design of Pedestrian Bridges*. Several additional constraints are placed on pedestrian bridges to ensure they are accessible, safe, and durable:

- 1) For guidance regarding determination of pedestrian bridge width, refer to Article 2.1.2 of this manual under **Shared-Use Paths and Pedestrian Walkways on Bridges**.
- 2) The maximum grade permitted on a pedestrian bridge is 8.33%. A grade flatter than the maximum is preferable. When the grade equals or exceeds 5%, provide a 5'-0" platform for each change in elevation of 2'-6". Also, a handrail is required when the grade equals or exceeds 5% per ADA requirements.
- 3) Protective screening, preferably a chain link fence system or a railing system, must be placed on both sides of the bridge where the bridge crosses roadway traffic. The height of the fence or railing system must be 8'-0" above the top of the sidewalk. For sites with special aesthetic treatments involving ornamental railings, a minimum height of 6'-0" will be allowed. Where a fence or railing system is also required on retaining walls that are connected to the bridge, it is recommended that the same system (chain link fence or railing) be utilized throughout; although heights may vary.
- 4) Provide a 6'-0" clear platform at the bottom of each ramp.
- 5) Provide a platform at each abrupt change in a horizontal direction. The minimum plan dimension for a platform is 5'-0" by 5'-0".
- 6) Lay out the profile grade such that there are no abrupt grade breaks at expansion joint devices. This will ensure that the cover plate does not bind on the deck or stick up above the deck. If a grade break is required, locate the expansion joint device completely to one side of break or the other.
- 7) Only in the rare case where handicap accessibility need not be provided can stairs be incorporated into a design. When stairs are provided, use the following guidelines:

- a) Provide stairs with a 1'-0" tread and a 6" rise.
  - b) Adjust the sidewalk or superstructure elevations to make all risers 6" tall.
  - c) The preferred number of risers in a flight of stairs is 14 to 16. The maximum number is 19.
- 8) Detail the rails in accordance with the following:
- a) Refer to Section 13 of this manual for metal railing height and spindle spacing requirements.
  - b) When required, place handrails 2'-8" above the top of the deck.
- 9) Provide an electrical ground for continuous chain link fences, ornamental railings, and metal handrails. If appropriate, provide bicycle ramps on pedestrian bridges that contain stairs.

### **Materials**

Use steel, prestressed concrete, reinforced concrete, or timber for the superstructure of pedestrian bridges. Aluminum is not an acceptable material for use in any portion of the superstructure.

The minimum structural steel thickness is  $\frac{1}{4}$  inch for pipe or tube sections and  $\frac{5}{16}$  inch for all other sections. The minimum thickness requirements do not apply to railings. Provide structural tubing details that are watertight or designed such that moisture cannot be trapped in or on the member to accelerate corrosion.

Use a high performance concrete mix for the deck of a pedestrian bridge. Contact the Regional Bridge Construction Engineer to determine which mix is most appropriate for the site: 3YHPC-M or 3YLCHPC-M.

The Brazilian hardwood known as IPE, though very durable, is not an accepted decking material on state or federally funded projects. If the use of IPE wood is desired by the owner, local funds are the only option for payment. In addition, a maintenance agreement is required to identify repair and replacement thresholds.

Stay-in-place deck forms for prefabricated steel truss spans are acceptable with District/Metro maintenance concurrence.

### **Bridge Substructure**

Use reinforced concrete supported on piling, drilled shafts, or spread footings for bridge substructures as recommended in the Bridge Construction Unit Foundation Recommendations report. Incorporate drainage systems (Detail B910) into the abutments as needed.

**Bridge Superstructure**

To limit transverse deck cracking due to negative flexure, provide additional longitudinal bars in the top of the deck over the piers. Stagger the ends of the additional longitudinal bars to transition the capacity of the section. (See Figures 9.2.1.8 and 9.2.1.9.)

Detail anchorages for the piers and abutments to resist uplift and overturning forces associated with wind loads.

Provide a cover plate over all pedestrian bridge expansion joint openings to protect pedestrians from a tripping hazard.

Type 5.0 strip seals with expansion joint openings up to 5.0 inches are allowed on pedestrian bridges since the joint is concealed by a cover plate.

**Highway Geometrics**

Meet MnDOT design standards for horizontal and vertical clearances for a pedestrian bridge over a roadway.

**2.4.1.3 Temporary  
Bridges and  
Widening****Temporary Bridges**

Temporary bridges are used to detour traffic while removal of an existing bridge and construction of a new bridge occur on the mainline of the roadway.

Design temporary bridges in accordance with the LRFD Specifications using the HL-93 live load with an associated load factor of 1.75.

For work zones with posted speeds of 45 mph or less, the bridge railing, bridge railing-to-deck connection, and deck overhang must meet the requirements of Test Level 2 or higher. For speeds greater than 45 mph, Test Level 3 is the minimum standard.

Use of glued-laminated (glulam) wood panels with a bituminous wearing course, where the glulam panels are placed transversely across beam supports, is an option for temporary bridge decks that has been highly successful. Follow the guidance below for this bridge deck type:

- Design glulam panels to meet AASHTO LRFD requirements. See BDM Article 8.2 for guidance on timber bridge decks and BDM Article 8.7.4 for a glulam panel design example.
- Use laminates made of visually graded Southern Pine (Identification Number 48) for glulam panels. The minimum net finished panel depth allowed is 5 inches.

**[Table 8.4.1.2.3-2]**

- For the bridge railing, use temporary portable precast concrete barriers (Standard Plate 8337) anchored to the glulam panels per Detail B919. The bridge railing, bridge railing-to-deck connection, and glulam panel overhang will meet the requirements of MASH Test Level 3 when anchored in accordance with Detail B919 and the following bullets. Overhangs that do not meet the limits below require a special design.
  - The maximum overhang allowed is 7 feet, measured from the centerline of beam to the edge of the glulam panel deck. In addition, the maximum distance from the centerline of fascia beam to the back bottom edge of the portable barrier is 3 feet.
  - For the bituminous wearing course, the minimum thickness allowed at the gutterline is 2 inches and the maximum is 4 inches. The portable barriers must be supported by wood plank risers with a thickness that matches the bituminous wearing course thickness at the gutterline. This ensures the barrier is at the correct height from the roadway to meet the test level requirements.
  - Loading is limited to the self-weight of the overhang components shown in B919, plus a maximum construction load of 0.020 ksf applied to the area between the back bottom edge of the portable barrier and the edge of deck.
  - Follow guidance provided in MnDOT *Memo to Designers #2019-01* and the MnDOT *Temporary Barrier Guidance Manual* regarding the required deflection distance behind the portable barriers to the edge of deck.

### **Temporary Widening**

Temporary widening occurs when staging requires widening of an existing bridge while construction of an adjacent new bridge occurs.

Design structural components of the temporary widening to meet or exceed the capacity of the existing bridge components. For a temporary widening, match the deck material of the existing bridge.

For temporary widening projects, design the barriers, the barrier/deck connection, and the deck overhang to meet the barrier test level required for the roadway.

#### **2.4.1.4 Bridge Approaches**

In most cases, the bridge approach panel will be included with the roadway grading plans for a project. Guidance for the treatment and details of approach panels can be found in the following:

#### Bridge Approach Treatment

The approach treatment standard sheets describe the limits and treatment of excavation and backfill near the abutments. These sheets are found in the *MnDOT Standard Plans Manual*, Figures 5-297.233 and 5-297.234. The Preliminary Bridge Plan contains a note indicating which approach treatment sheet to use.

#### Bridge Approach Panel

The standard sheets covering bridge approach panels are found in the *MnDOT Standard Plans Manual*, Figures 5-297.223 through 5-297.228. These figures cover standard approach panels for abutments with joints, abutments without joints, abutments with different amounts of skew, different mainline pavement types, and miscellaneous details. The Bridge Preliminary Plan contains a note indicating which approach panel sheets to use.

Specify a concrete wearing course on approach panels when the bridge deck has a concrete wearing course. The wearing course on the approach panels will be placed at the same time as the wearing course on the bridge. Include the approach panel wearing course quantity in the summary of quantities for the superstructure.

#### **2.4.1.5 Survey**

When assembling the survey sheets for final plans, verify that the most current grading plans are being used.

Include the centerlines and object lines for the abutment and pier footings on the final design survey sheets. Also identify and locate all test piles.

#### **2.4.1.6 Utilities**

The Bridge Office Preliminary Plans Unit in coordination with the District Traffic Engineer determines if structural provisions must be made for safety lighting (roadway, navigation, inspection, etc.), signing, or signals. Coordination is also done with the MnDOT District Project Manager regarding the need for other types of utilities.

#### **2.4.1.6.1 Suspended Utilities & Utilities Embedded in Bridges**

The conduit for utilities is to be suspended below the deck on hanger systems between the beams. Locate the entire conduit and hanger system above the bottom of the beams and generally below the diaphragms or in the lower openings of a cross frame diaphragm. To minimize the impact to the structure in the future, avoid casting conduits for utility companies in the deck, sidewalk, or barriers/parapets.

Use polyvinyl chloride (PVC) coated hot dipped galvanized rigid steel conduit (RSC) for utilities requiring conduit. Use galvanized steel hangers and supports.

Roadway lighting conduit (1<sup>1/2</sup> inch diameter maximum) will be allowed in barriers/parapets (maximum of 2 per barrier/parapet) and sidewalks.

Suspended water, sewer, communications, and electrical power (less than 35kV) utility systems are allowed on bridges. However, natural gas pipelines are considered a safety risk and will not be allowed.

For hanging conduit systems on bridges with parapet or semi-integral type abutments, and when conduit is embedded in concrete barrier, deck, or sidewalk, use a combination expansion/deflection fitting at the abutments. This will accommodate horizontal movements (due to temperature change, creep, shrinkage, etc.) and vertical movements (due to jacking operations for bearing replacement, etc.).

For hanging systems on bridges with integral abutments, only an expansion fitting is required at the abutments.

The temperature movements of RSC approximate those of concrete. Consequently, lateral bracing is not needed. Choose a transverse spacing for the conduits that permits proper placement of concrete between embedded anchors.

Typical conduit and utility details are available in the Bridge Office final design cell library, available at:

<http://www.dot.state.mn.us/bridge/drafting-aids.html>

#### **2.4.1.6.2 Buried Utilities**

To protect structures, restrictions on the location of new or existing buried utilities and drainage pipes must be considered near new and existing bridge substructures, box culverts, and wall structures (e.g.- conventional concrete cantilever retaining walls, sheet pile walls, mechanically stabilized earth (MSE) walls, etc.).

Location restrictions, installation techniques, protection measures, and plan review are required for utilities in the utility-critical region immediately adjacent to and below the structure, defined below for the type of structure. Allowance of new or existing utilities in the utility-critical region of a new or existing structure requires review and approval from the MnDOT Bridge Office. Additional restrictions on the location of utilities may be specified in other documents relevant to the project.

For purposes of this section, utilities are defined as any utility requiring a permit as well as State owned utilities and stormwater structures. Dry utilities are defined as facilities that do not carry fluid or gasses, such as power and communications. Wet utilities are defined as those facilities that carry fluid or gasses, except they do not include roadway edge drains or subsurface drains associated with the bridge or wall structure.

Certain types of utilities may pose a significant risk to foundations when placed in the utility-critical region. If these types of utilities were to fail, the foundation could be at risk of failure due to the loss of material from localized scour or erosion. The determination of high-risk utilities will be made on a case-by-case basis by the Bridge Office and will be based on many factors including, but not limited to utility location, flow pressure, flow rate, structure size, and utility size. Additional restrictions to those contained in this document could be applied to utilities that pose a significant risk to the foundations.

**Utilities Near Bridge Substructures and Conventional Concrete Cantilever Retaining Walls Supported on Spread Footings, Prefabricated Modular Block Walls (PMBWs) Without Earth Reinforcement, and Box Culverts**

The limits of the utility-critical region are defined as 50 feet lateral to, 50 feet below, and the distance from the bottom of footing elevation or leveling pad to the highest point of ground (up to 15 feet maximum).

Within the utility-critical region, three zones have been identified to provide general guidance for MnDOT approval. See Figure 2.4.1.6.2.1 for the definition of the zones.

The following restrictions on utilities are dependent on their position relative to the structure:

Zone ①

For new substructures/walls/box culverts, new utilities are to be placed and existing utilities relocated outside of Zone ①. For existing substructures/walls/box culverts, new utilities are to be placed outside of Zone ①. If this is impractical or impossible, the requirements for utilities in Zone ① are:

- General
  - All new pipes and conduits must be designed for any surcharge loading due to structure bearing pressures and possible resulting deformations. All existing pipes and must be checked for structural adequacy.

- Future open trench excavation for utility access is prohibited to protect the substructure, wall, or box culvert from potential undermining. Other forms of excavation may be permitted in this zone with Bridge Office approval.
- New or Existing Dry Utilities
  - Dry utilities may be located transversely or longitudinally (i.e., in any direction between parallel and perpendicular to the substructure or wall) in Zone ①.
  - Utility owners may choose to case dry utilities to allow for future maintenance or access. However, casing is not required.
- New or Existing Wet Utilities
  - Wet utilities may be located transversely (defined as perpendicular to the substructure or wall, or up to 30 degrees from perpendicular) in Zone ①.
  - Wet utilities may not be located longitudinally (defined as parallel to the substructure or wall, or up to 60 degrees from parallel) in Zone ①.
  - All wet utilities in Zone ① require pipe with gaskets that meet Standard Plate 3006 for concrete culverts and storm drains, or joints designed to prevent leakage due to pressurized flow for other utilities or pipe materials.
  - All wet utilities in Zone ① must be cased for additional protection against risk of leakage and soil loss. Refer to the *MnDOT Utility Accommodation & Coordination Manual*, Article VII.E, for casing requirements. If facilities are too large or cannot be cased effectively, a risk analysis approved by the Regional Bridge Construction Engineer and a site-specific design is required.

#### Zone ②

The requirements for utilities in Zone ② are:

- General
  - Future excavation for maintenance or replacement will be permitted with proper sheeting and shoring. No unbraced open cuts will be allowed.
- New or Existing Dry Utilities
  - Dry utilities may be located transversely or longitudinally (i.e., in any direction between parallel and perpendicular to the substructure or wall) in Zone ②.
  - Utility owners may choose to case dry utilities to allow for future maintenance or access. However, casing is not required.
- New or Existing Wet Utilities

- Wet utilities may be located transversely or longitudinally (i.e., in any direction between parallel and perpendicular to the substructure or wall) in Zone ②.
- All wet utilities in Zone ② require pipe with gaskets that meet Standard Plate 3006 for concrete culverts and storm drains, or joints designed to prevent leakage due to pressurized flow for other utilities or pipe materials.
- All wet utilities in Zone ② must be cased for additional protection against risk of leakage and soil loss, with the exception of some stormwater facilities. Casing is required for stormwater pipes with any of the following:
  - Flow velocities greater than 10 ft/s, where flow rate is determined for maximum velocity condition.
  - Pipe diameters or spans greater than 48 inches.Other stormwater facilities need not be cased in Zone ② unless required by contract specifications or as recommended by the Bridge Office. Refer to the *MnDOT Utility Accommodation & Coordination Manual*, Article VII.E, for casing requirements. If facilities are too large or cannot be cased effectively, a risk analysis approved by the Regional Bridge Construction Engineer and a site-specific design is required.

#### Zone ③

The only requirement for new or existing utilities in Zone ③ is that all wet utilities require pipe with gaskets that meet Standard Plate 3006 for concrete culverts and storm drains, or joints designed to prevent leakage due to pressurized flow for other utilities or pipe materials.

If the conditions above cannot be met, options include relocation or replacement of the utility or placing the substructure or wall on deep foundations (piles or drilled shafts). However, note that there are some restrictions for wet utilities placed in Zone ① of deep foundations. Also note that deep foundations are not an option for PMBWs or box culverts.



### **Utilities Near Bridge Substructures and Conventional Concrete Cantilever Retaining Walls Supported on Deep Foundations (Piles or Drilled Shafts)**

Note that there is a risk of interference between the piles/shafts and utilities that is dependent on the pile/shaft locations, size of the utility, and the order of construction. Coordination is required between the designer of the structure, the road designer, and the utility owner to avoid any potential interference.

There are two utility-critical regions, defined in Figure 2.4.1.6.2.2, for substructures and walls on deep foundations. The left side is for end bearing piles and the right side for piles that do not extend to bedrock.

#### Zone ①

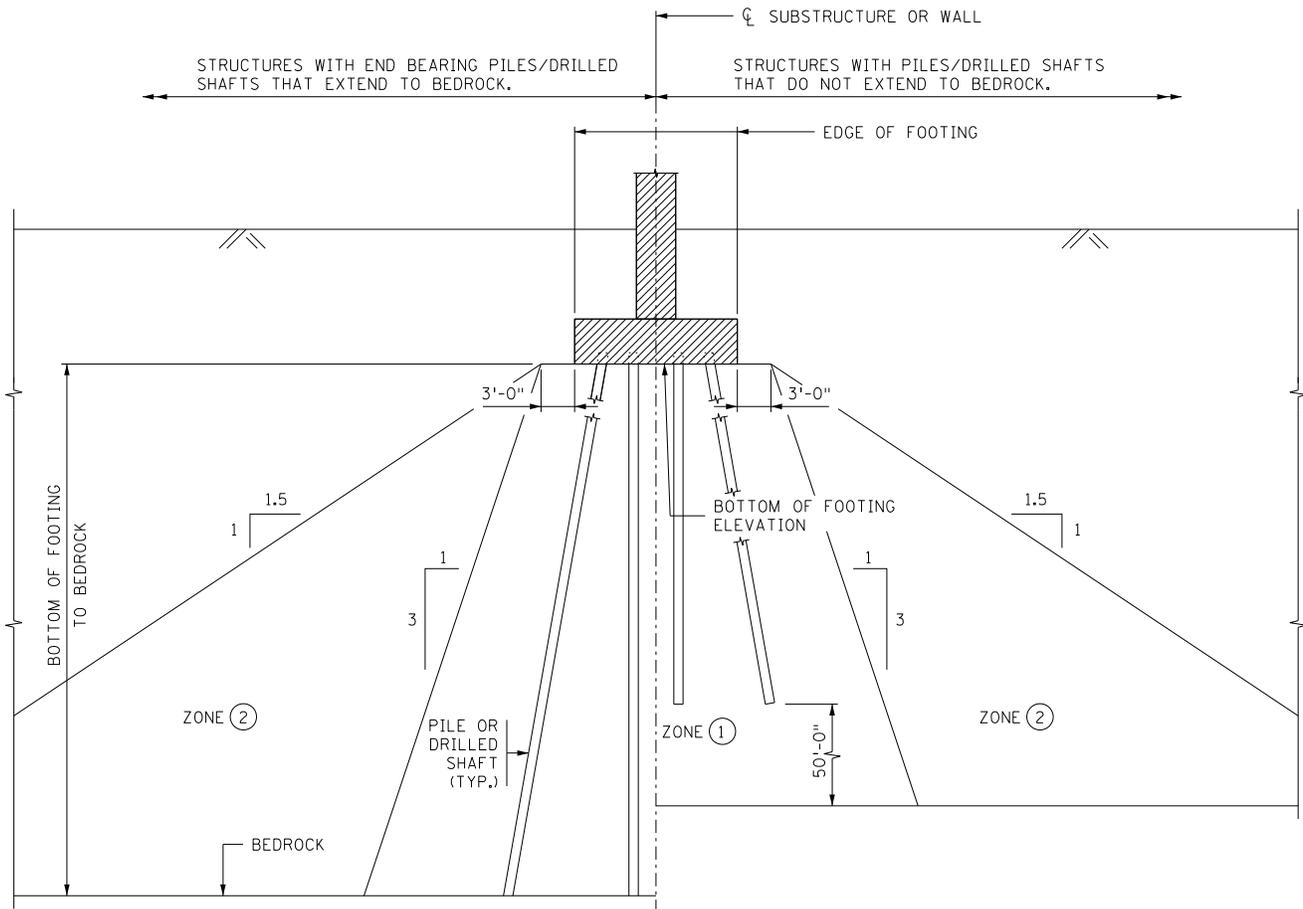
The requirements for utilities in Zone ① are:

- General
  - Future open trench excavation for utility access is prohibited to protect the substructure or wall from potential undermining. Other forms of excavation may be permitted in this zone with Bridge Office approval.
- New or Existing Dry Utilities
  - Dry utilities may be located transversely or longitudinally (i.e., in any direction between parallel and perpendicular to the substructure or wall) in Zone ①.
  - Utility owners may choose to case dry utilities to allow for future maintenance or access. However, casing is not required.
- New or Existing Wet Utilities
  - Wet utilities may be located transversely or longitudinally (i.e., in any direction between parallel and perpendicular to the substructure or wall) in Zone ①.
  - All wet utilities in Zone ① require pipe with gaskets that meet Standard Plate 3006 for concrete culverts and storm drains, or joints designed to prevent leakage due to pressurized flow for other utilities or pipe materials.
  - All wet utilities in Zone ① must be cased for additional protection against risk of leakage and soil loss. Refer to the *MnDOT Utility Accommodation & Coordination Manual*, Article VII.E, for casing requirements. In lieu of casing, a risk analysis approved by the Regional Bridge Construction Engineer is acceptable for substructures on deep foundations. The risk analysis must include a sensitivity study that examines the deep foundation stability and capacity if soil loss should occur.

Zone ②

The requirements for utilities in Zone ② are the same as for Zone ①, except that for wet utilities, casing is not required.

There are no utility restrictions outside of Zone ②.



**Figure 2.4.1.6.2.2**

**Utility-Critical Region for Utilities Near Bridge Substructures and Conventional Concrete Cantilever Retaining Walls Supported on Deep Foundations**

### **Utilities Near Mechanically Stabilized Earth (MSE) Walls and Prefabricated Modular Block Walls (PMBWs) With Earth Reinforcement**

For new and existing MSE walls and PMBWs, the limits of the utility critical region are defined as 50 feet lateral to the front of the wall, 50 feet measured laterally beyond the end of the earth reinforcement, 50 feet below the bottom of the leveling pad, the top of the fill on the front side of the wall, and the top of the reinforced earth/retained fill in the back of the wall. Refer to Figure 2.4.1.6.2.3.

Five zones have been identified to provide general guidance for MnDOT approval when new or existing utilities are near MSE walls and PMBWs constructed using earth reinforcement. The zones are defined in Figure 2.4.1.6.2.3.

In addition to the requirements below, walls near utilities where stray electrical current could be an issue are subject to the following restrictions. For existing walls with earth reinforcement consisting of metal strips, new buried high voltage lines or other utilities/infrastructure that may cause stray currents in the soil mass must be located a minimum of 20 feet outside the limits of Zone ⑤. For new walls where new or existing buried high voltage lines or other utilities/infrastructure that may cause stray currents in the soil mass are located closer than 20 feet to the limits of Zone ⑤, use of non-metallic geosynthetic earth reinforcement is required.

The restrictions on utilities are dependent on their position relative to the structure:

#### Zones ①, ②, and ③

Zones ①, ②, and ③ restrictions for MSE walls and PMBWs are the same as for bridge substructures on spread footings. See restrictions given previously for these zones found under **Utilities Near Bridge Substructures and Conventional Concrete Cantilever Retaining Walls Supported on Spread Footings, Prefabricated Modular Block Walls (PMBWs) Without Earth Reinforcement, and Box Culverts.**

#### Zone ④

The requirements for utilities in Zone ④ are as follows:

- General
  - When excavating within this zone, install shoring or sheet piling (without damage to the geomembrane), to protect the reinforced zone.

- Future excavation for maintenance or utility replacement will be permitted with proper sheeting and shoring. No unbraced open cuts will be allowed.
- New or Existing Dry Utilities
  - Dry utilities may be located transversely or longitudinally (i.e., in any direction between parallel and perpendicular to the wall) in Zone ④.
  - Casing is not required in Zone ④.
- New or Existing Wet Utilities
  - Wet utilities may be located transversely or longitudinally (i.e., in any direction between parallel and perpendicular to the wall) in Zone ④.
  - All wet utilities in Zone ④ require pipe with gaskets that meet Standard Plate 3006 for concrete culverts and storm drains, or joints designed to prevent leakage due to pressurized flow for other utilities or pipe materials.
  - Casing is not required in Zone ④.

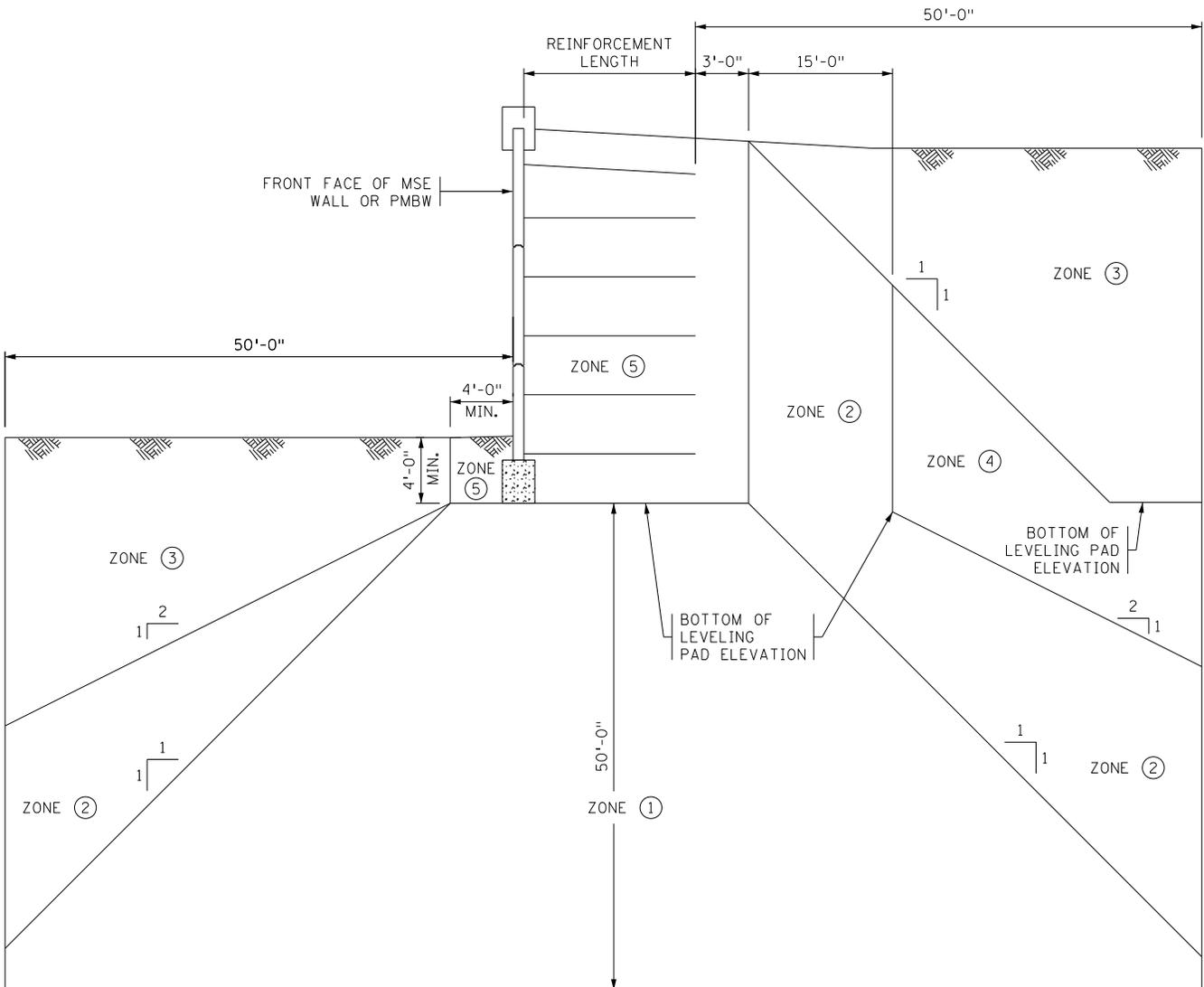
#### Zone ⑤

Existing utilities in Zone ⑤ cannot remain in place during construction of new MSE walls and PMBWs as they will be disturbed. New utilities are to be placed outside of Zone ⑤. If this is impractical or impossible, the requirements for utilities in Zone ⑤ are as follows:

- General
  - Future excavation is prohibited in this zone to protect the structural integrity of the wall.
- New Dry Utilities
  - Dry utilities may be located transversely or longitudinally (i.e., in any direction between parallel and perpendicular to the wall), but only during the original construction of the wall.
  - Utility owners may choose to case dry utilities to allow for future maintenance or access. However, casing is not required for dry utilities.
- New Wet Utilities
  - Wet utilities may be located transversely (defined as perpendicular to the wall, or up to 30 degrees from perpendicular), but only during the original construction of the wall.
  - Wet utilities may not be located longitudinally (defined as parallel to the wall, or up to 60 degrees from parallel) in Zone ⑤.
  - All wet utilities in Zone ⑤ require pipe with gaskets that meet Standard Plate 3006 for concrete culverts and storm drains, or

joints designed to prevent leakage due to pressurized flow for other utilities or pipe materials.

- All wet utilities in Zone ⑤ must be cased for additional protection against risk of leakage and soil loss. Refer to the *MnDOT Utility Accommodation & Coordination Manual*, Article VII.E, for casing requirements. If facilities are too large or cannot be cased effectively, a risk analysis approved by the Regional Bridge Construction Engineer and a site-specific design is required.



**Figure 2.4.1.6.2.3**

**Utility-Critical Region for Utilities Near Mechanically Stabilized Earth (MSE) Walls and Precast Concrete Block Walls (PMBWs) With Earth Reinforcement**

**Utilities Near Sheet Pile Walls, Soldier Pile Walls, and Noise Walls**

For new and existing sheet pile walls, soldier pile walls, and noise walls, the limits of the utility-critical region are defined as 50 feet lateral to each edge of pile/post, 30 feet below the pile/post tip, and the vertical distance above the pile/post tip to the ground elevation on the each side of the wall.

Within the utility-critical region, three zones have been identified to provide general guidance for MnDOT approval. See Figure 2.4.1.6.2.4 for the definition of the zones.

Zone ①

For new walls, new utilities are to be placed or existing utilities relocated outside of Zone ①. For existing walls, new utilities are to be placed outside of Zone ①. If this is impractical or impossible, the requirements for utilities in Zone ① are as follows:

- General
  - Future excavation is prohibited in this zone for sheet pile and soldier pile walls to protect the structural integrity of the wall. Limited future excavation is allowed for utility access for noise walls with proper consideration of post support and adequate bracing.
- New or Existing Dry Utilities
  - Dry utilities may not be located transversely (i.e., defined as perpendicular to the wall, or up to 30 degrees from perpendicular) to sheet pile walls in Zone ①. Dry utilities may be located transversely to soldier pile and noise walls (defined as perpendicular to the wall, or up to 30 degrees from perpendicular), provided they meet the minimum clearances to the piles/posts shown in the partial wall elevation of Figure 2.4.1.6.2.4.
  - Dry utilities may be located longitudinally (defined as parallel to the wall, or up to 60 degrees from parallel) to sheet pile walls, soldier pile walls, or noise walls, but only before the original construction of the wall.
  - All dry utilities must be cased in Zone ① for sheet pile and soldier pile walls to allow for future maintenance or access. Casing is not required for noise walls. Refer to the *MnDOT Utility Accommodation & Coordination Manual*, Article VII.E, for casing requirements.
- New or Existing Wet Utilities
  - Wet utilities may not be located transversely (defined as perpendicular to the wall, or up to 30 degrees from perpendicular) to sheet pile walls in Zone ①. Wet utilities may be located transversely to soldier pile and noise walls (defined

as perpendicular to the wall, or up to 30 degrees from perpendicular), provided they meet the minimum clearances to the piles/posts shown in the partial wall elevation of Figure 2.4.1.6.2.4.

- Wet utilities may not be located longitudinally (defined as parallel to the wall, or up to 60 degrees from parallel) to sheet pile walls, soldier pile walls, or noise walls in Zone ①.
- All wet utilities in Zone ① require pipe with gaskets that meet Standard Plate 3006 for concrete culverts and storm drains, or joints designed to prevent leakage due to pressurized flow for other utilities or pipe materials.
- All wet utilities must be cased in Zone ① for soldier pile walls to provide additional protection against risk of leakage and soil loss. Casing is not required for noise walls. Refer to the *MnDOT Utility Accommodation & Coordination Manual*, Article VII.E, for casing requirements. If facilities are too large or cannot be cased effectively, a risk analysis approved by the Regional Bridge Construction Engineer and a site-specific design is required.

#### Zone ②

For new walls, new utilities are to be placed and existing utilities relocated outside of Zone ②. For existing walls, new utilities are to be placed outside of Zone ②. If this is impractical or impossible, the requirements for utilities in Zone ② are:

- General
  - Future open trench excavation for utility access is prohibited to protect the wall from potential undermining. Other forms of excavation may be permitted in this zone with Bridge Office approval.
- New or Existing Dry Utilities
  - Dry utilities may be located transversely or longitudinally (i.e., in any direction between parallel and perpendicular to the wall) in Zone ②.
  - Utility owners may choose to case dry utilities to allow for future maintenance or access. However, casing is not required.
- New or Existing Wet Utilities
  - Wet utilities may be located transversely (defined as perpendicular to the wall, or up to 30 degrees from perpendicular) in Zone ②.
  - Wet utilities may not be located longitudinally (defined as parallel to the wall, or up to 60 degrees from parallel) in Zone ②.

- All wet utilities in Zone ② require pipe with gaskets that meet Standard Plate 3006 for concrete culverts and storm drains, or joints designed to prevent leakage due to pressurized flow for other utilities or pipe materials.
- All wet utilities must be cased in Zone ② for sheet pile and soldier pile walls to provide additional protection against risk of leakage and soil loss. Casing is not required for noise walls. Refer to the *MnDOT Utility Accommodation & Coordination Manual*, Article VII.E, for casing requirements. If facilities are too large or cannot be cased effectively, a risk analysis approved by the Regional Bridge Construction Engineer and a site-specific design is required.

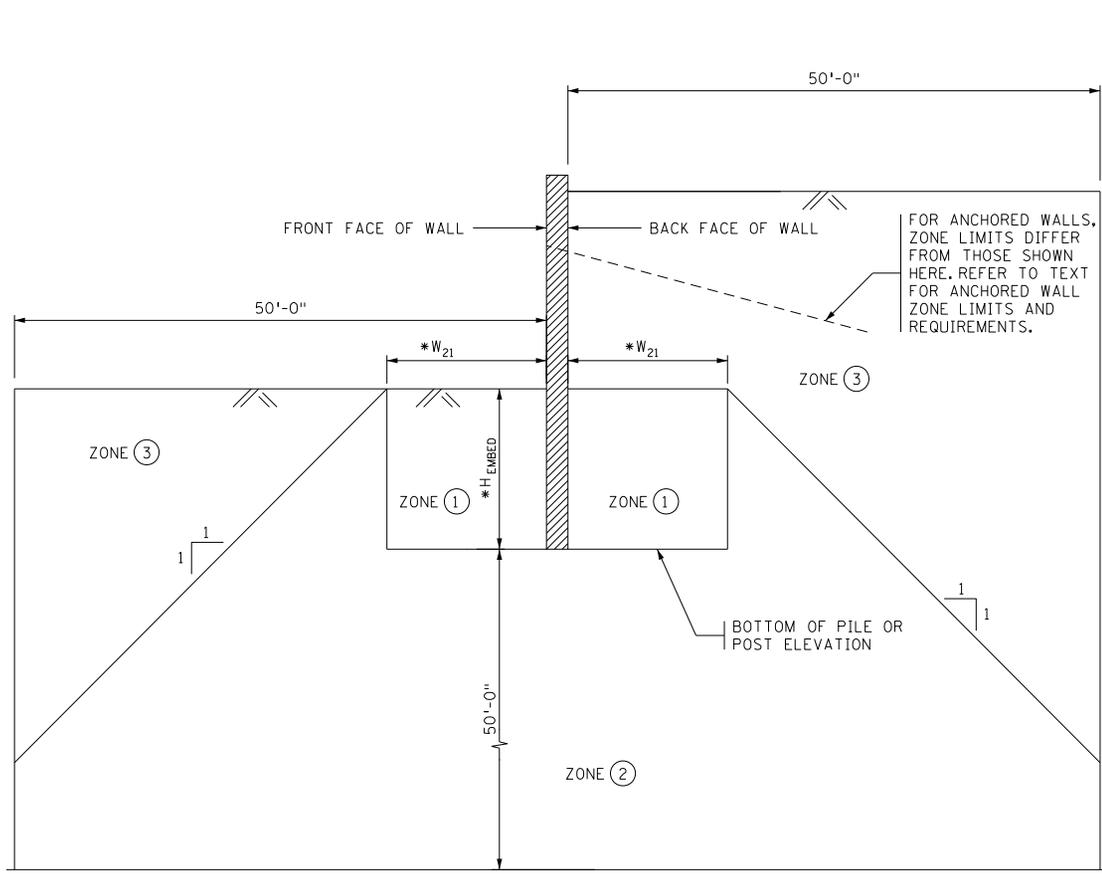
### Zone ③

The only requirement for utility installations in Zone ③ is that all wet utilities require pipe with gaskets that meet Standard Plate 3006 for concrete culverts and storm drains, or joints designed to prevent leakage due to pressurized flow for other utilities or pipe materials.

An exception to this is for anchored walls, where anchors extend from the front face of the wall into the fill behind the wall. For this case, replace the boundary and requirements for Zone ③ behind the anchored wall with the boundaries and requirements for Zone ⑤ and Zone ② outlined under **Utilities Near Mechanically Stabilized Earth (MSE) Walls and Prefabricated Modular Block Walls (PMBWs) With Earth Reinforcement.**

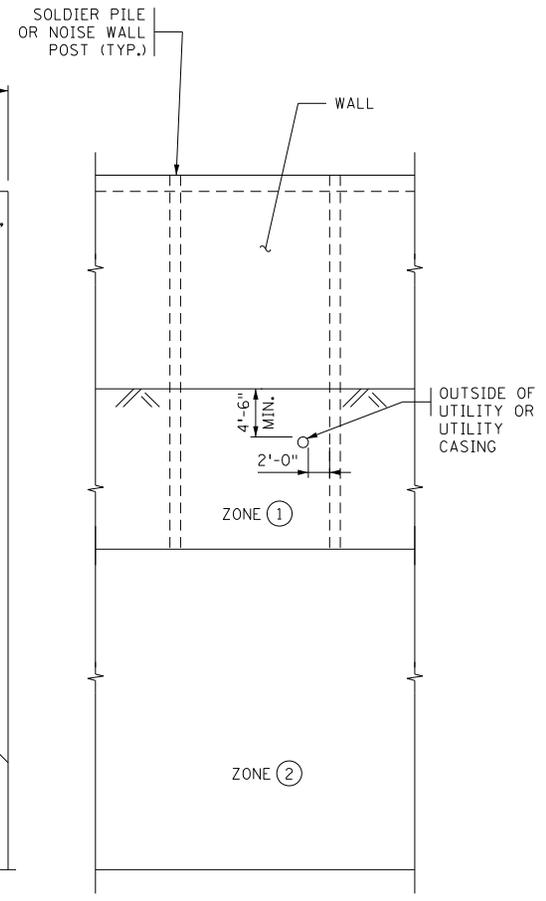
**Utility-Critical Region for Utilities Near Sheet Pile Walls, Soldier Pile Walls, and Noise Walls**

**Figure 2.4.1.6.2.4**



\* H<sub>EMBED</sub> = DISTANCE FROM BOTTOM OF PILE ELEVATION TO GROUNDLINE ELEVATION ON FRONT OF WALL  
 W<sub>21</sub> = H<sub>EMBED</sub>

SECTION THRU WALL



PARTIAL ELEVATION OF SOLDIER PILE WALL OR NOISE WALL

**2.4.1.7 Precedence  
of Construction  
Documents**

Designers, while striving to produce accurate error-free construction documents, may at times end up with documents that have conflicting content. A hierarchy has been established to determine which content is governing for a project. In general, the more project specific the document, the higher the document's position in the hierarchy. Section 1504 of the *Standard Specifications for Construction* describes the precedence of construction documents for a project:

If discrepancies exist between the Contract documents, the following order of precedence applies:

- 1) Addenda
- 2) Special Provisions
- 3) Project-Specific Plan Sheets
- 4) Supplemental Specifications
- 5) Standard Plan Sheets and Standard Plates
- 6) Standard Specifications

If discrepancies exist between dimensions in the Contract documents, the following order of precedence applies:

- 1) Plan dimensions
- 2) Calculated dimensions
- 3) Scaled dimensions

**2.4.1.8 Design  
Calculation  
Requirements**

Office practice is to permit the limit states to be exceeded by a maximum of 3%. However, caution should be exercised to ensure that a 3% exceeded limit state at a particular location does not adversely affect the structure load rating.

**2.4.2 Final Plans**

The plan order shall typically follow this list:

- General Plan and Elevation
- Cross Section and Pay Items
- Staging Plan
- Working Point Layout
- Removal Details
- Abutment Details and Reinforcement
- Pier Details and Reinforcement
- Framing Plan
- Beam Details
- Superstructure Details and Reinforcement
- Plan Details (Railing, Expansion Joint, Slope Paving, Conduit, etc.)
- B-Details
- As-Built Bridge Data

- Surveys
- Borings
- Unrevised Informational Sheets

When presenting geometric information, enough baseline information needs to be provided to permit others to verify the information presented. For example, the top of roadway elevations presented on a bridge layout sheet can be confirmed by others using vertical curve information on the general elevation and the cross slopes provided on the typical transverse section.

In general, do not present the same dimensions several times. Providing dimensions in multiple locations increases the chance that not all dimensions will be updated as changes occur during the design process.

The clarity of the details used in plan sets should be a primary concern of designers. Only the simplest details should combine the presentation of concrete geometry and reinforcement. In most cases there is less confusion if two details are used, one to convey concrete geometry and a second to identify and locate reinforcement.

Show the initials of the individuals responsible for the design, drafting, design check, and drafting check on all plan sheets except the as-built sheet, survey sheets, boring sheets, and unrevised information sheets (such as those showing alignment tabulations, superelevation transitions, or aesthetics) taken from the preliminary bridge plan. Note that these unrevised informational sheets are to be placed at the end of the bridge plan. Only include informational sheets with direct relation to the bridge plan and remove any project wide informational sheets only pertaining to other bridges. For the boring sheets, show the initials of the individuals responsible for the drafting and drafting check. Similarly, all sheets, except the as-built sheet, survey sheets, boring sheets, and unrevised information sheets must be certified by a Professional Engineer licensed in the State of Minnesota.

In most cases, details are presented with stationing increasing as one moves from the left side to the right side of the sheet. Always include a north arrow on plan views. Plan views are typically oriented with north arrows pointing toward the top or to the right of the sheet. Stationing increases for northbound and eastbound traffic.

#### **2.4.2.1 Drafting Standards**

The Bridge Office has adopted standards to be used when drafting plan sheets. Download *Summary of Recommended Drafting Standards* from: <http://www.dot.state.mn.us/bridge/drafting-aids.html>

### **2.4.2.2 Drafting Guidelines**

#### **Sheet Layout and Continuity**

Read plans from a contractor's perspective to check that they contain all information needed to build the bridge. Make sure enough dimensions are given for constructability. Use extra details for uncommon work. Use perspective views when clarity is needed.

Use sheets efficiently. Balance the drawings on sheets to avoid one sheet being empty while another is crowded. Use additional sheets, as needed, to avoid crowding details on sheets. Make sure that details, data, and other information given on more than one sheet agree between sheets. Avoid unnecessary repetition of details and notes.

Large-scale corner details are required for all skewed bridges and for other complex corners.

Round dimensions to the nearest  $\frac{1}{8}$  of an inch.

Note and dimension bar splices.

Cross-referencing sheets to details is recommended.

Use bill of reinforcement tables for all but very minor reinforced concrete work. Do not enlarge details (such as rebar bends) just to fill up space. Referencing bar bend details by letter to various generic shapes should never be used.

Keep details together for abutments, piers, superstructure, etc.

For abutments, piers, and other complex drawings, use different views and sections to separate dimensions and reinforcement.

Place pile design loads and notes pertaining to a particular substructure on the sheet that contains the footing plan view.

For bridges with numerous footings and curved alignment, a separate foundation layout drawing is recommended.

If the plan contains numerous variable dimensions and other data (especially for framing plans and beams), make use of tables to keep this data in order.

On the Framing Plan, show bearing type beside each bearing point instead of lines and arrows, which tend to clutter the drawing.

For simple beam spans (prestressed beams, etc.), dimension beam spacing at pier cap along centerline of the pier(s). Include supplemental dimensions along centerline of bearing for curved and flared structures.

On projects with staged construction, use enough drawings to clearly indicate how the bridge construction is to be coordinated with the staging. Keep structure units together. Reinforcement and quantity tabulations are to be split between stages.

On repair projects, clearly indicate cut lines and extent of all removals. If there is a saw cut, be sure to use a straight line (WT=5). If elevations are taken off original plans, note as such and require the contractor to verify elevations in the field.

When it appears that plan notes, such as procedure descriptions, specifications, etc., will become excessively wordy, relegate these notes to the special provisions.

List general notes first and specific numbered notes last. Number specific detail notes with circles and reference the detail to which they apply. Place all notes together on the right hand side of the sheet.

Leave extra lines in the Summary of Quantities and Bill of Reinforcement for additions. Also, leave extra space in the list of notes.

### **Pay Quantities**

Make computations neat and readable. Strive for continuity. These computations may be needed for future reference and the reader must be able to interpret them.

Box in or underline computation totals for quicker take off. Initial, date, and put the bridge (or project) number on every computation sheet.

Two sets of independently worked quantity computations are required for each pay item.

Arrange design and quantity computations into a neat and orderly package.

In addition to pay item quantities, compute informational quantities. This applies to final plans for new bridges and bridge repair plans for existing bridges. Do not include these quantities in the bridge plan, but instead submit them to the Bridge Estimating Unit for use in developing the Engineer's Estimate. Examples of information quantities include: summary quantities for conduit systems, summary quantities for drainage systems,

cubic yard quantities for concrete items paid for by the square foot or linear foot, and miscellaneous minor items such as polystyrene and waterproofing membrane. Sample forms for reporting informational quantities are available at: <http://www.dot.state.mn.us/bridge/design.html>

### **Additional Drafting Guidance**

Additional drafting resources can be found at:

<http://www.dot.state.mn.us/bridge/drafting-aids.html>

These resources include:

- *Suggested Reinforcement Detailing Practices*
- *Guidelines for Checking Final Design Bridge Plans*
- Bridge Preliminary and Final Design Seed Files
- Bridge Preliminary and Final Design Cell Libraries

### **2.4.2.3 General Plan and Elevation**

The General Plan and Elevation sheet is intended to summarize the primary features and horizontal geometry of the bridge. Figure 2.4.2.3.1 shows an example General Plan and Elevation sheet and Figure 2.4.2.3.2 shows a Typical Cross Section sheet with pay items.

#### **Plan**

On the plan view identify the following: working points, working line, centerlines, utilities, location of in-place bridges or substructures, ditch drains, deck drains, lights, and nameplate. Label the following: span lengths, deck width, size of angles between the working line and centerlines, horizontal curves, minimum horizontal clearance to substructure units, point of minimum vertical clearance for each roadway under the bridge, extent of slope protection, roadway stationing and elevations, and distance between twin bridges. Provide a north arrow. Tie bridge dimensions to working points. Show the direction of traffic for each design lane.

#### **Elevation**

Present the primary vertical geometry of the bridge on the elevation view. This consists of vertical curve data, end slopes, existing ground lines, footing elevations, limits of excavation, grading notes, ditch clean out along railroad tracks, and scale. Label bearings as fixed, expansion, or integral. Also label piers, spans, abutments, and slope protection.

For bridges over waterways, provide hydraulic information. Required information includes: channel bottom width, low member elevation, design high water elevation, and assumed flowline elevation.

For grade separation bridges, provide the minimum vertical and horizontal clearances. In addition, provide the dimension from centerline of pier to toe of slope protection. If there is no side pier, give the dimension from toe of slope to centerline of roadway. Dimension the pier, lane, and shoulder widths on the roadway under. Lane slopes on the roadway under are typically omitted, but can be provided if space permits.

When illustrating slope protection use a straight slope line; do not follow the ditch radius curve. To reduce confusion concerning slopes, do not show slopes as 1:2. Many individuals are unsure of whether the first or second number is the horizontal part of the slope. Show the slopes graphically. Where slopes need to be provided in text, explicitly call out the slopes (e.g., 1V: 2H).

### **Typical Cross Section**

The typical cross section is the third general view of the structure. Combined with the general plan and elevation views, the primary geometry of the bridge is conveyed. On the typical cross section show transverse bridge dimensions, lane widths and slopes, beam depth and spacing for all spans, roadway slab and concrete wearing course thicknesses, type of barrier, medians, sidewalks, profile grade location, working line, and all centerlines.

For staged construction projects, provide the in-place, interim, and final cross sections, including temporary anchored or unanchored safety barrier locations.

For complex projects, consider creating a separate plan sheet for pay items and notes for clarity.

### **Utilities**

Show all utilities that may affect bridge construction. Note what is to be done with them (will they be moved, will they no longer be used or do they need to be protected during construction).

### **Miscellaneous**

Provide a Design Data block on the General Plan and Elevation Sheet of the bridge plan set. The information given in the block provides a summary of the primary parameters used for the design. Information in the Design Data block includes: design specifications, design live load, design material properties, future wearing course load assumed in the design, deck area, traffic data, and the operating rating for the new structure. See Appendix 2-C.

Also on this sheet, identify the governing standard specifications for construction. Show a north arrow on the plan view and include a block for engineering certification. Present applicable project numbers on the first sheet; project numbers depend on specific funding sources, so there may be both state and federal project numbers.

Review the title block to ensure it accurately describes the bridge. Within the title block provide span lengths to the nearest foot and the bridge type identification number. The three-character identification number should follow the numbering scheme provided in Appendix 2-A of this manual.

Include any additional standard construction notes and the sheet list for the plan set on the first sheet of the plan set. Provide the schedule of quantities for the entire bridge in tabular form on the second or third sheet of the plan.

Standard practice for placement of bridge nameplates and bench marks is as follows:

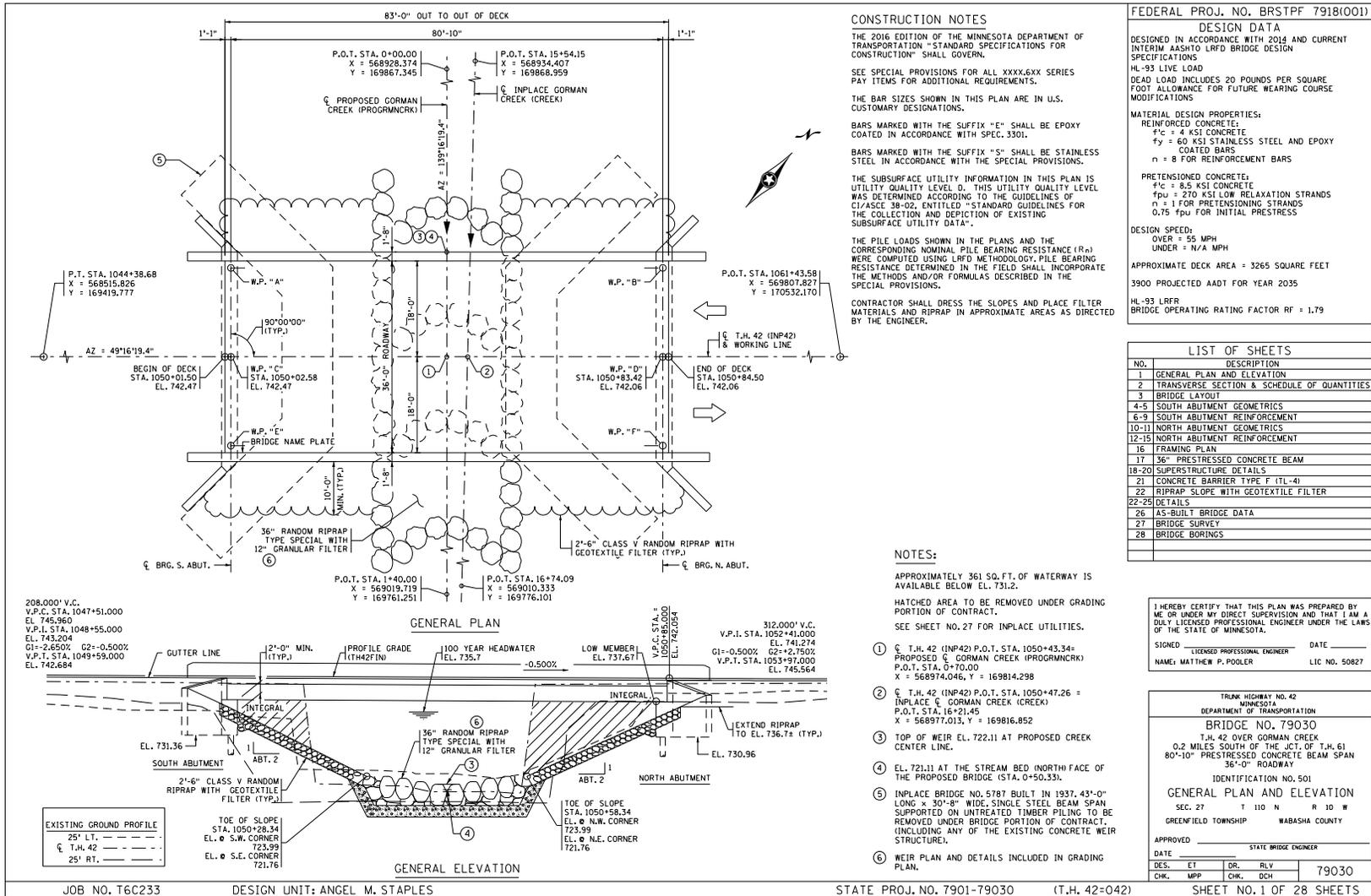
- Generally, include a single nameplate on each bridge with its own bridge number, attaching it to the traffic barrier/parapet in the right hand corner as one approaches the bridge. Specifically, for two-way bridges with a roadway running north and south, place the nameplate in the southeast corner. For a roadway running east and west, provide the nameplate at the northeast corner. On twin bridges (two one-directional bridges that are side-by-side, but each with their own bridge number), place a nameplate on each right hand corner approaching each bridge. Exceptions to the above include railroad, timber, pedestrian, and boardwalk bridges, where the nameplate is to be placed on a substructure unit. On bridges that are widened, redecked, or that receive rail modifications that result in additional roadway width, install a new nameplate with the original year completed and the year renovated.
- Do not show a bench mark disk in the bridge plan. Placement of bench mark disks on bridges is no longer needed.

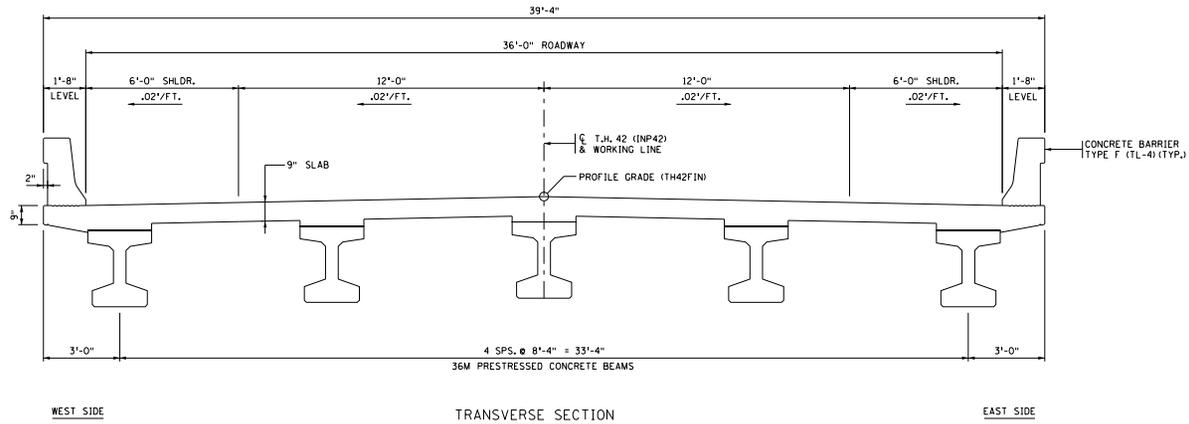
Check if ditch drainage pipe is necessary for the project. If drainage pipe is necessary and the contract has multiple portions (grading, bridge, etc.), identify which portion of the contract contains the pipe. Label ditch drainage pipe on plan and elevation views.

Concrete or aggregate slope protection is used along a highway or railway (grade separation structures). Aggregate slope protection is used more

frequently when pedestrian traffic below the bridge is limited. Stream crossings use riprap slope protection supported on a granular or geotextile filter. The Preliminary Bridge Plan will indicate the type of slope protection to be used.

Figure 2.4.2.3.1  
General Plan and Elevation





SCHEDULE OF QUANTITIES FOR ENTIRE BRIDGE

ITEM NO.	ITEM	UNIT	QUANTITY
2104.601	REMOVE REGULATED WASTE MATERIAL (BRIDGE)	LUMP SUM	1
2401.501	STRUCTURAL CONCRETE (3852)	CU. YD.	82 (P)
① 2401.513	TYPE F (TL-4) BARRIER CONCRETE (3552)	LIN. FT.	194 (P)
② 2401.541	REINFORCEMENT BARS (EPOXY COATED)	POUND	33500 (P)
2401.541	REINFORCEMENT BARS (STAINLESS-60KSI)	POUND	460 (P)
2401.601	STRUCTURE EXCAVATION	LUMP SUM	1
2401.618	BRIDGE SLAB CONCRETE (3YHPC-W)	SQ. FT.	3265 (P)
2402.590	ELASTOMERIC BEARING PAD	EACH	10
2405.502	PRESTRESSED CONCRETE BEAMS 36M	LIN. FT.	411 (P)
2405.511	DIAPHRAGMS FOR TYPE 36M PREST BEAMS	LIN. FT.	34 (P)
③ 2442.501	REMOVE EXISTING BRIDGE	LUMP SUM	1
2452.519	C-1-P CONCRETE TEST PILE 85 FT LONG 12"	EACH	1
2452.519	C-1-P CONCRETE TEST PILE 100 FT LONG 12"	EACH	1
2452.527	PILE REDRIVING	EACH	2
2452.603	C-1-P CONCRETE PILING 12"	LIN. FT.	1020
2502.502	DRAINAGE SYSTEM TYPE (B910)	LUMP SUM	1
2511.501	RANDOM RIPRAP CLASS V	CU. YD.	407 (P)
2511.515	GEOTEXTILE FILTER TYPE VII	SQ. YD.	734 (P)

- NOTES:
- ① QUANTITY INCLUDES THAT PORTION FOR THE APPROACH PANELS.
  - ② QUANTITY INCLUDES THAT PORTION FOR BARRIER REINFORCEMENT ON THE APPROACH PANEL.
  - ③ REMOVE INPLACE BRIDGE NO. 5787 (INCLUDES THE CONCRETE SPILLWAY).

Figure 2.4.2.3.2  
Typical Cross Section

CERTIFIED BY	DATE	TITLE:	DES:	ET	DR:	RLV	APPROVED:	BRIDGE NO. 79030
NAME: MATTHEW P. POOLER	LIC. NO. 50827	TRANSVERSE SECTION AND SCHEDULE OF QUANTITIES	CHK:	MPP	CHK:	DCH	SHEET NO. 2 OF 28 SHEETS	

#### **2.4.2.4 Bridge Layout and Staking Plan**

The Bridge Layout Sheet is used by surveyors to locate the bridge in space with its primary geometry. The primary geometry consists of centerline of roadway(s) and centerline of substructure bearings. Working points are located on substructure bearing centerlines where they are intersected by fascia beam lines and working lines. By providing stationing, X-coordinates, and Y-coordinates for each of the working points, the position of the bridge can be fixed. Figure 2.4.2.4.1 contains an example.

In Figure 2.4.2.4.1, the working line and its azimuth are labeled. Also shown is the angle of intersection between the working line and each of the substructure units and roadways under the bridge. As a primary geometry line, the working line should be labeled throughout the plan set.

Place the control point at the intersection of the survey line and centerline of cross road, track etc. For river crossings, place the control point at an abutment centerline of bearing. Label the control point with its coordinates. Coordinates of the control point and the working points should be given to three decimals of a foot. Tie the working point layout to the control point. Present dimensions in feet (a note on the sheet should say the same).

List the coordinates for all working points in a table labeled "DIMENSIONS BETWEEN WORKING POINTS". Stations and the distances between working points should be presented to the nearest 0.01 foot. Coordinates are assumed to be given in the Minnesota State Plane Coordinate System. If another system is used, place a note on the sheet identifying the system used.

In addition to horizontal geometry, a limited amount of vertical geometry is provided on the Bridge Layout Sheet. The vertical geometry consists of elevations and drops. The elevation at the top of roadway and the bridge seat is provided for all working points located on beam lines and is appended to the "DIMENSIONS BETWEEN WORKING POINTS" table.

Drop or elevation difference information is provided for each substructure unit. Drop information is summarized in the "TOP OF ROADWAY TO BRIDGE SEAT" table. The table should contain the following items:

- 1) Deck Thickness
- 2) Stool Height
- 3) Beam Height
- 4) Bearing Height
- 5) Total Height

If the drop dimension is the same for all beam lines, provide a single value for each substructure unit. If the drop dimensions vary at substructure locations, provide a value for each beam line. Total values should be given in both inches and decimals of a foot to two places.

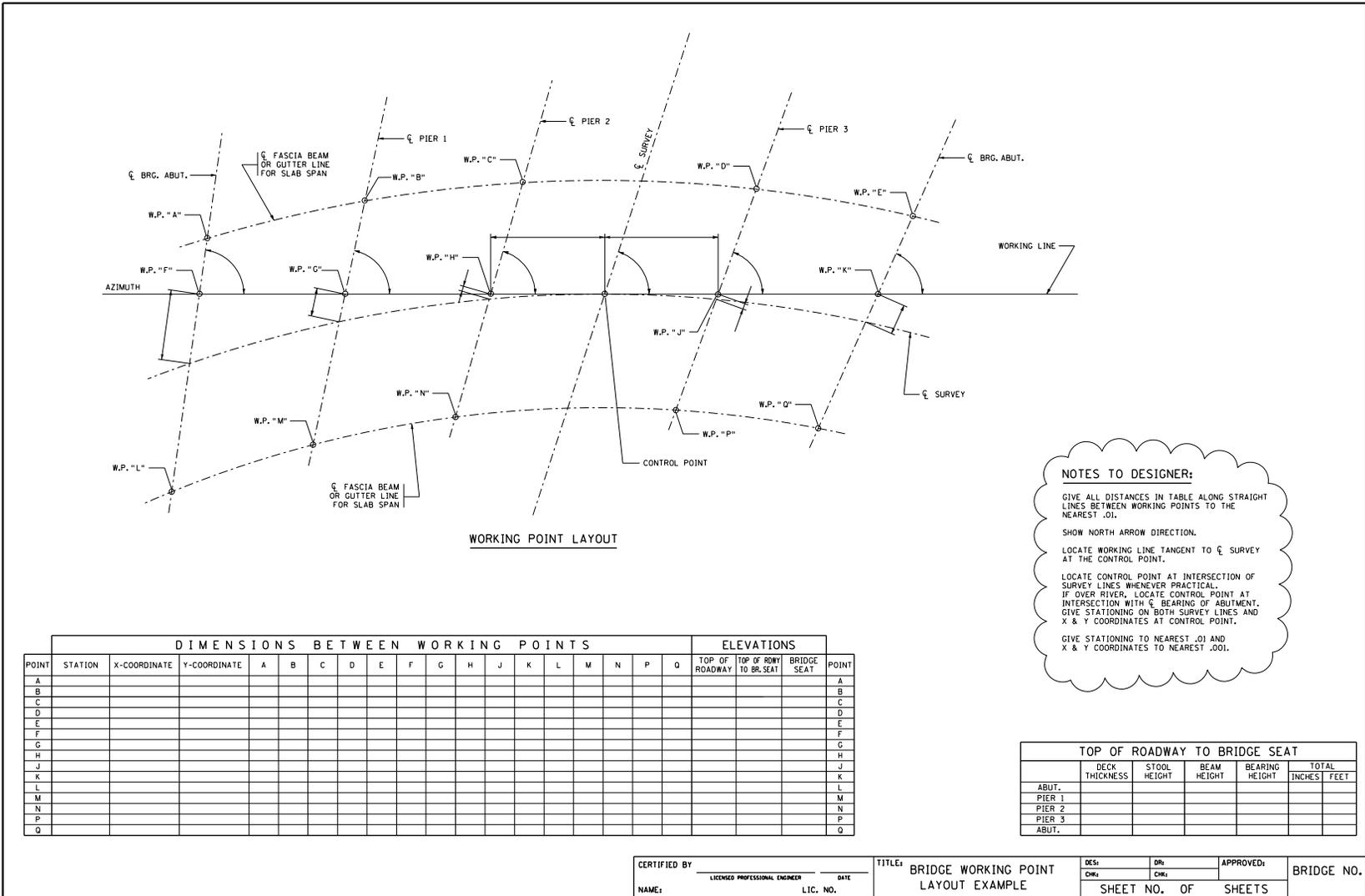


Figure 2.4.2.4.1 Bridge Layout

**2.4.2.5 Standard Abbreviations**

Use standard abbreviations to clarify information on plan sets and reduce the clutter on a crowded plan sheet. Appendix 2-B presents a list of standard abbreviations that can be utilized in a plan. Define abbreviations used in a plan set on the sheet where they are used or as part of a General Notes sheet.

**2.4.2.6 Inclusion of Standard Bridge Details and Bridge Standard Plans in Plan Sets**

There are two parts to the Standard Bridge Details Manual: Part I and Part II. They are published on the Bridge Office Web site at:

<http://www.dot.state.mn.us/bridge/standards.html>

Bridge details are intended, where applicable, to be incorporated into a set of bridge plans.

Bridge Details Manual Part I, typically referred to as the B-Details, are presented in a "portrait" orientation on an 8 1/2" x 11" sheet. Included are details for nameplates, pile splices, bearings, diaphragms, steel superstructures, floor drains, and other miscellaneous details.

Bridge Details Manual Part II consists of details that occupy an entire plan sheet. The majority of these details are for barriers, parapets, medians, prestressed concrete beams, and expansion joints.

Similar to Bridge Details Manual Part II, details from the Bridge Standard Plans Manual are intended to be incorporated into bridge plan sets and occupy an entire plan sheet. The information presented may be much more in-depth as the information for multiple designs is presented on a single sheet. Bridge Standard Plans are only available for precast concrete box culverts.

How standard details are incorporated into a bridge plan will depend on the amount of revisions needed to the details, falling into 3 categories:

- 1) Standard is unmodified: This is when the standard is used as drawn with no changes, or with all blanks filled in where expected. Box out and place hatching across all specific details that do not apply. Do not remove them. It is not necessary to cross out alternative sizes in tables or alternate dimensions that are not used, or circled notes that are only referred to in the details that do not apply.
- 2) Standard is modified: This is when details, dimensions, or notes on the standard have been specifically modified from what is shown in the standard. In this case, place the word "MODIFIED" under the B-Detail or after the Figure Number. Also add a box containing a note stating what was modified to help plan readers quickly locate them. Do not "cloud" the changes.

- 3) Standard is substantially modified: This is when much of the standard is changed by extensive and/or numerous modifications to details, dimensions, and notes to the extent that it ceases to reflect the standard. It will require a judgment call on the part of the engineer as to when this category applies. In this case, remove the State Bridge Engineer approval block and the figure number. Note that for this case, the engineer takes full responsibility for the details shown on the sheet.

#### **2.4.2.7 Standard Plan Notes**

Similar to other plan elements, standard plan notes have been prepared to increase the consistency of information presented on final design plans. Plan notes serve a variety of purposes; they communicate design criteria, specific construction requirements, and a variety of notes pertaining to the construction or fabrication of specific bridge elements. Appendix 2-C contains the Standard Plan Notes. These notes have been grouped into the following categories:

- Design Data and Projected Traffic Volumes
- Construction Notes
- Signature Block
- Drainage and Erosion Control
- Excavation and Earthwork
- Reinforcement
- Foundations
- Steel Materials, Fabrication, and Erection
- Concrete Placements
- Welded Steel Bearing Assemblies
- Cutting and Removal of Old Concrete
- Joints and Joint Sealer
- Timber Bridges
- Miscellaneous

Designers unfamiliar with MnDOT's Standard Plan Notes should review the list prior to beginning final design. Reviewing the notes prior to design will familiarize designers with the material properties to be used, and other constraints typically placed on construction. Perform a second review of the notes at the end of design to ensure that all applicable notes were incorporated into the plan set.

#### **2.4.2.8 Quantity Notes and Pay Items**

#### **Standard Summary of Quantities Notes**

During construction, contractors are compensated according to the work they complete. The value of the work item is identified when the contractor submits their bid. For each work item or pay item the contractor must supply a price. The pay items are coordinated with the *MnDOT Standard Specifications for Construction* and the project special provisions. To clarify what is included in a specific pay item, the Bridge Office has assembled a Standard Summary of Quantity Notes. Like other plan elements, these notes help ensure uniformity across plan sets and permit MnDOT to generate a historical price database that can be used to estimate the cost of future bridges. The Standard Summary of Quantities Notes for bridge projects is listed in Appendix 2-D.

#### **Pay Items**

The current MnDOT list of pay items (commonly referred to as the Trns\*port List) is located at the following link:

<https://transport.dot.state.mn.us/Reference/refItem.aspx>

When populating the "SCHEDULE OF QUANTITIES FOR ENTIRE BRIDGE" in the bridge plan, it is important to note the following:

- 1) The Trns\*port List shows a 12 digit number under the "ITEM NUMBER" column. Provide only the first seven numbers including the decimal point (all the numbers before the /) for each pay item in the bridge plan.
- 2) The Trns\*port List shows two descriptions for each pay item. Historically, the "ITEM SHORT DESCRIPTION" column was used for pay item descriptions in the bridge plan. Beginning with bridge projects governed by the 2020 Edition of the *MnDOT Standard Specifications for Construction*, the "ITEM LONG DESCRIPTION" column is now used as the pay item description in all bridge plans.
- 3) Use the "PLAN UNIT NAME" column from the Trns\*port List for listing the units in the bridge plan.
- 4) The item number, item description, and units must read exactly as shown in the Trns\*port List, including spaces, dashes, parentheses, etc. In order to avoid errors, it is suggested to copy and paste them directly from the list into the bridge plan.

For each pay item shown in the plan, provide a reconciled quantity estimate. Some pay items are to be designated as "plan quantity pay items", for which payment to the contractor will be based on the quantity given in the plan rather than measuring in the field. For these items, include a "(P)" as an appendix to the item label. For example:

2401.507 STRUCTURAL CONCRETE (3B52) 699 CU YD (P)

Pay items are to be designated as “plan quantity pay items” when the quantity for payment can be calculated using the dimensions given in the plan and the dimensions are not expected to change in the field. Some examples include:

- structural concrete paid for by the cubic yard
- bridge slab concrete paid for by the square foot
- reinforcement bars paid for by the pound
- prestressed concrete beams paid for by the linear foot
- ornamental metal railing paid for by the linear foot

Some examples of pay items that are **not** “plan quantity pay items” include:

- piling paid for by the linear foot (because plan quantity is an estimate only and final pile lengths are determined in the field)
- random riprap paid for by the cubic yard (because plan dimensions are approximate and actual volume may differ)
- removal and patching of concrete slabs paid for by the square foot (because actual area will be determined in the field)
- structural excavation paid for by lump sum (because it is not a calculated value based on plan dimensions)
- bearing assemblies paid for by the each (because it is not calculated using dimensions from the plan)

For minor work items (membrane waterproofing system, bit. felt, joint filler, etc.) that are not official “pay items”, avoid using the term “incidental” in plan notes. Instead, tie minor work items to a specific pay item using the term “included in”, which will help those in the field to know who is responsible for it. For example: PAYMENT FOR MEMBRANE WATERPROOFING SYSTEM INCLUDED IN ITEM “BRIDGE SLAB CONCRETE (3YHPC-M)”.

### **Miscellaneous**

Round off quantities to the nearest pay item unit except for the following:

- Earth excavation to nearest 10 cubic yards.
- Reinforcement bars and structural steel to nearest 10 pounds.
- Piling lengths to nearest 5 feet.

When computing concrete quantities, consider the following:

- For CIP piles that are embedded in a crash strut or an infill wall or a pile bent pier encasement wall, deduct the pile volume when determining the concrete quantity. For these cases, the embedded pile volume is considered substantial enough to affect the concrete quantity. For all other cases where piles are embedded in concrete (e.g. – H-piles embedded in a crash strut, CIP or H-piles embedded in

a footing, etc.), do not deduct the embedded pile volume when determining the concrete quantity. The embedded pile volume is minimal and considered unnecessary.

- Do not deduct the rebar volume when determining the concrete quantity.

When computing small bituminous quantities use the following:

Wearing course = 110 pounds / square yard / inch thickness

Shoulder or Wearing Course (6.5%)

$0.065$  (thickness in inches) (110 pounds) = \_\_\_ pounds / sq. yard

Tack Coat = 0.03 gallons / square yard

Binder or Base Course (5.3%)

$0.053$  (thickness in inches) (110 pounds) = \_\_\_\_ pounds / sq. yard

Compute deck area (rounded to the nearest square foot) by multiplying the transverse out-to-out bridge width by the longitudinal end-of-deck to end-of-deck distance. (Do not include bridge approach panels or paving brackets.)

When computing structural steel quantities, increase the calculated weight by 1.5% to account for the weight of steel for welds and bolt stick-through.

### **2.4.3 Revised Sheets**

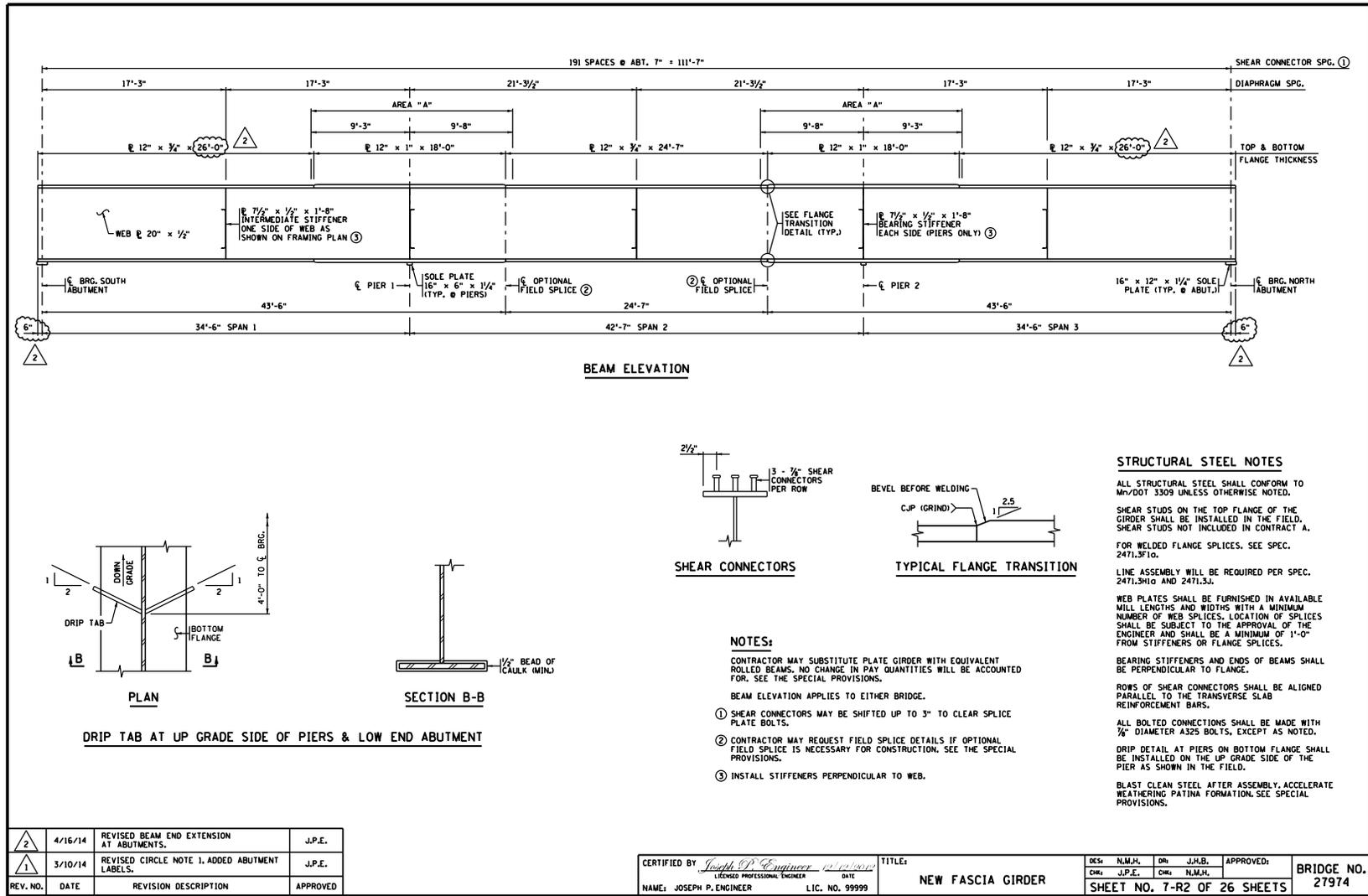
Sometimes, revisions to the plan are required after the letting due to an error found in the plan or other issues that arise during construction. When this occurs, use the following procedure:

1) Revise the sheet as follows (See Figure 2.4.3.1):

- i. Make the necessary revisions to the sheet.
- ii. In the revision block, provide the revision number within a triangle border, the revision date, a description of the revision, and the initials of the engineer who approved the revision. When sheets have been revised multiple times, add new entries to the revision block for each sheet revision. Do not remove the revision block entries for the previous revisions.
- iii. "Cloud" the actual revisions to the sheet and include the revision number within a triangle border next to the "clouded" change.
- iv. When sheets have been revised multiple times, remove revision "clouds", revision numbers, and triangles for the previous revisions (except as noted in ii. above). Only "cloud" the current revisions and include the current revision number within a triangle border.

- v. Change the sheet number by placing a "-R" and the revision number after the original sheet number. For example, revision 1 to sheet 7 will be designated "SHEET NO. 7-R1", revision 2 will be designated "SHEET NO. 7-R2", etc. For situations where an additional plan sheet must be inserted as part of the revision, repeat the preceding sheet number with an "A" after it. For example, as part of revision 1 where a sheet needs to be added between sheet 5 and 6, designate the new revised sheet as "SHEET NO. 5A-R1".
- 2) Plot and certify the revised sheet.
- 3) Draft a transmittal letter from the State Bridge Design Engineer to the Resident Engineer in the District construction office. Submit the letter and the revised sheet to the State Bridge Design Engineer for signature and distribution. Memo templates are available on the Bridge Office network drive. Consultants should contact MnDOT Bridge Office Project Manager to obtain the file.

Figure 2.4.3.1



---

**2.5 Reconstruction Guidelines and Details** Typical details for the reconstruction of barriers, superstructure joints, and pavement joints are presented in this section.

**2.5.1 Superstructure** [Future manual content]

**2.5.1.1 Barriers** [Future manual content]

**2.5.1.2 Wearing Course** [Future manual content]

**2.5.1.3 Expansion/Fixed Joints** There are many different existing joint configurations in service. The more prevalent configurations have been given "type" designations for pay item purposes. In order to promote consistency among repair plan details, guidance is provided as follows for each "type":

**Reconstruct Expansion Joint Type A**

Use when replacing an in-place waterproof joint with a new waterproof joint at a parapet type abutment where the joint is located at the front of the parapet. See Figure 2.5.1.3.1. For payment, use item no.:

2433.603 "RECONSTRUCT EXPANSION JOINT TYPE A", LIN. FT.

**Reconstruct Expansion Joint Type B**

Use when replacing an in-place waterproof joint with a new waterproof joint at a parapet type abutment where the joint is located at the back of the parapet. See Figure 2.5.1.3.2. For payment, use item no.:

2433.603 "RECONSTRUCT EXPANSION JOINT TYPE B", LIN. FT.

**Reconstruct Expansion Joint Type C**

This type is no longer used.

**Reconstruct Expansion Joint Type D**

Use when replacing an in-place waterproof joint with a new waterproof joint at a pier. See Figure 2.5.1.3.3 for a concrete superstructure and Figure 2.5.1.3.4 for a steel superstructure. For payment, use item no.:

2433.603 "RECONSTRUCT EXPANSION JOINT TYPE D", LIN. FT.

**Reconstruct Expansion Joint Type E**

Use when replacing an in-place waterproof joint with a new waterproof joint at a hinge. See Figure 2.5.1.3.5. For payment, use item no.:

2433.603 "RECONSTRUCT EXPANSION JOINT TYPE E", LIN. FT.

**Reconstruct Expansion Joint Type F**

Use when replacing an in-place finger joint or modular joint with a new finger joint and waterproof trough. See Figure 2.5.1.3.6. For payment, use item no.:

2433.603 "RECONSTRUCT EXPANSION JOINT TYPE F", LIN. FT.

**Reconstruct Expansion Joint Type Davidson**

Use when replacing an in-place waterproof joint located at the front of a parapet abutment with a new waterproof joint that includes an abutment end block which is integral with the approach panel.

See Figure 2.5.1.3.7 for the details. Depending upon the geometry of the end block and the condition of the in-place reinforcement, there may be permutations to the details and notes to include in the repair plan:

- If the abutment back face bars that extend into the end block cannot be salvaged, or if the in-place bars do not consist of at least #4 bars with a maximum spacing of 1'-0", addition of bars with adhesive anchors is required. Use #5 epoxy coated bars at a maximum spacing of 1'-0" for the added reinforcement.
- If the end block thickness is < 9", leave a 4" hook on the in-place back face bars. Alternatively, provide new #5 epoxy coated bars with a 4" hook that have a maximum spacing of 1'-0" and are attached to the abutment parapet wall with adhesive anchors.
- If the end block overhangs the main body of the abutment by more than 6" (not including the edge of deck regions), provide additional bars or support, or confirm that in-place reinforcement is adequate by using strut-and-tie analysis method.

The designer must evaluate the specific situation in conjunction with the Regional Bridge Construction Engineer and provide the appropriate notes and details.

Always include an adhesive anchor pay item in the bridge plan. If the expectation is that no anchors will be needed, include a pay quantity equal to 10% of the existing abutment backwall back face bars.

Also note that the bridge designer must contact the road designer to coordinate the approach panel length and limits.

For payment of all work excluding the adhesive anchors, use item no.:

2433.603 "RECONSTRUCT EXP JOINT TYPE DAVIDSON", LIN. FT.

For payment of the adhesive anchor work, use item no.:

2433.603 "ANCH TYPE REINF BARS (TYPE NT)", EACH

### **Reconstruct Expansion Joint Type Modular**

Use when replacing an in-place finger joint or modular joint with a new modular joint. See Figure 2.5.1.3.8. For payment, use item no.:

2433.603 "RECONSTRUCT EXPANSION JOINT TYPE MODULAR", LIN. FT.

### **Reconstruct Expansion Joint Type Special**

Use for joint reconstructions that do not match any of the types given above. For payment, use item no.:

2433.603 "RECONSTRUCT EXPANSION JOINT TYPE SPECIAL", LIN. FT.

### **Reconstruct Fixed Joint**

Use when replacing an in-place waterproof joint at a contraction abutment with a new fixed joint, where the in-place approach panel is also being replaced. For this joint type, the approach panel is removed and replaced in the grading portion of the contract. The joint reconstruction work only includes the work on the bridge side of the joint. See Figure 2.5.1.3.9. For payment of all work excluding the adhesive anchors, use item no.:

2433.603 "RECONSTRUCT FIXED JOINT", LIN. FT.

For payment of the adhesive anchor work, use item no.:

2433.603 "ANCH TYPE REINF BARS (TYPE NT)", EACH

### **Reconstruct Fixed Joint Type A**

Use when replacing an in-place waterproof joint at a contraction abutment with a new fixed joint, where the in-place approach panel is not being replaced. For this joint type, the approach panel is partially removed and replaced in the bridge portion of the contract. The joint reconstruction work includes the work done on both the bridge side and approach panel side of the joint. See Figure 2.5.1.3.10. For payment of all work excluding the adhesive anchors, use item no.:

2433.603 "RECONSTRUCT FIXED JOINT TYPE A", LIN. FT.

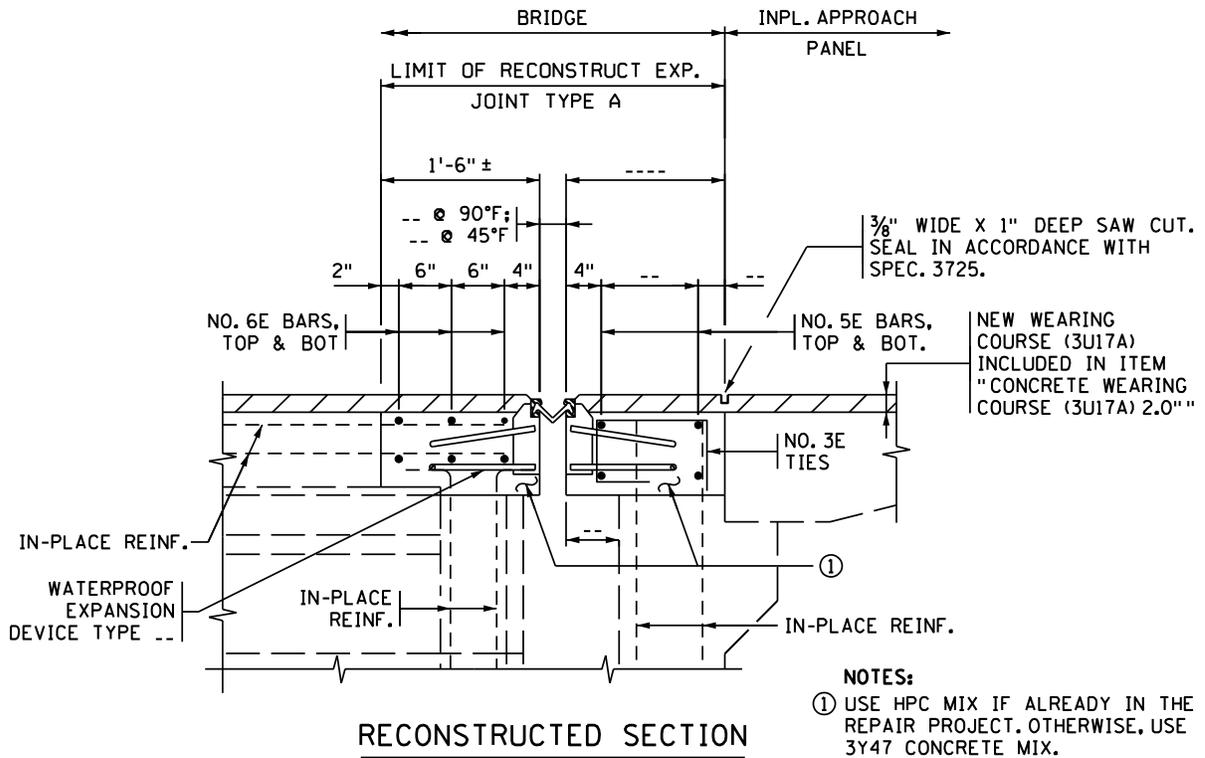
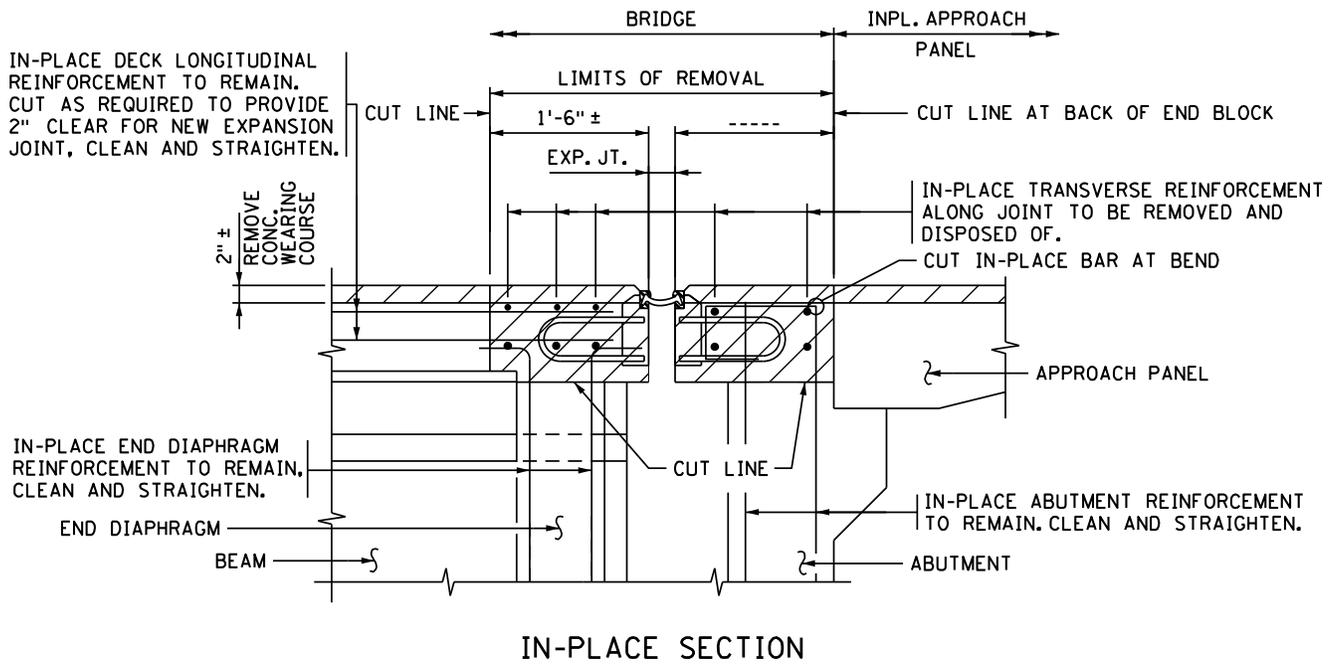
For payment of the adhesive anchor work, use item no.:

2433.603 "ANCH TYPE REINF BARS (TYPE NT)", EACH

**Reconstruct Fixed Joint Type B**

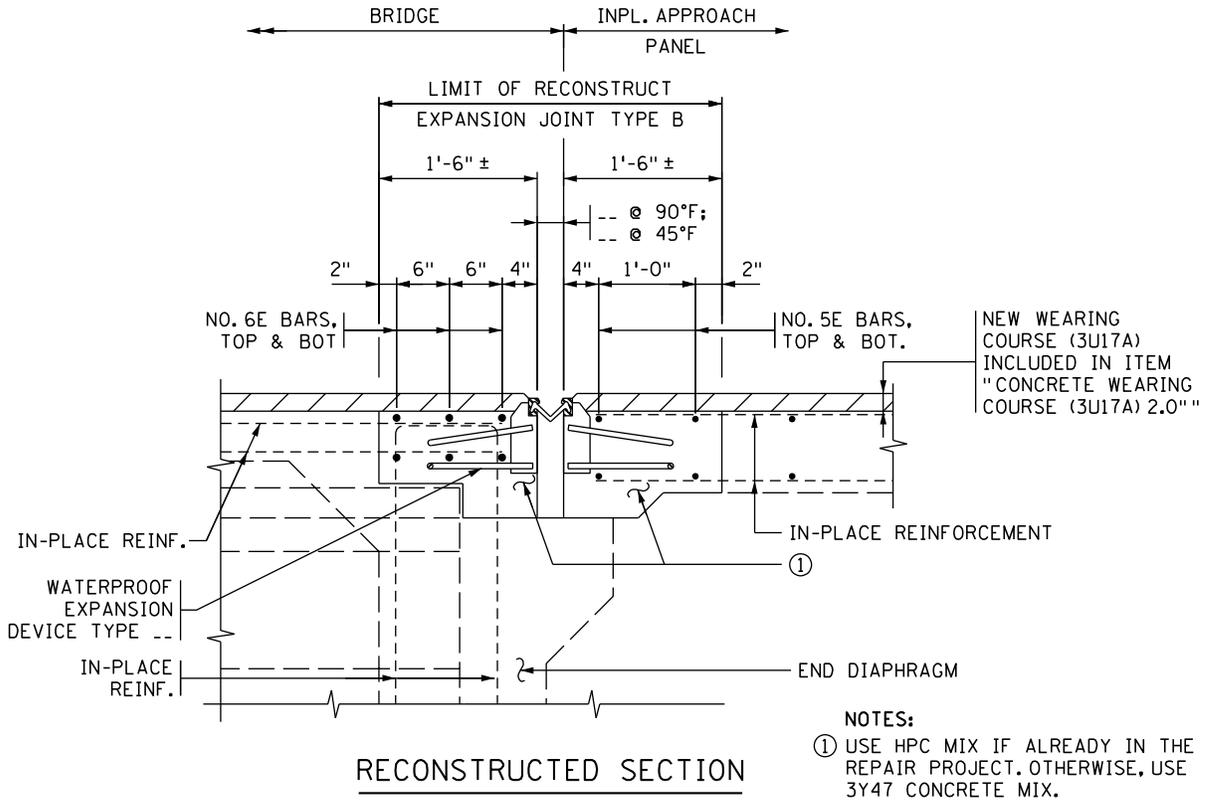
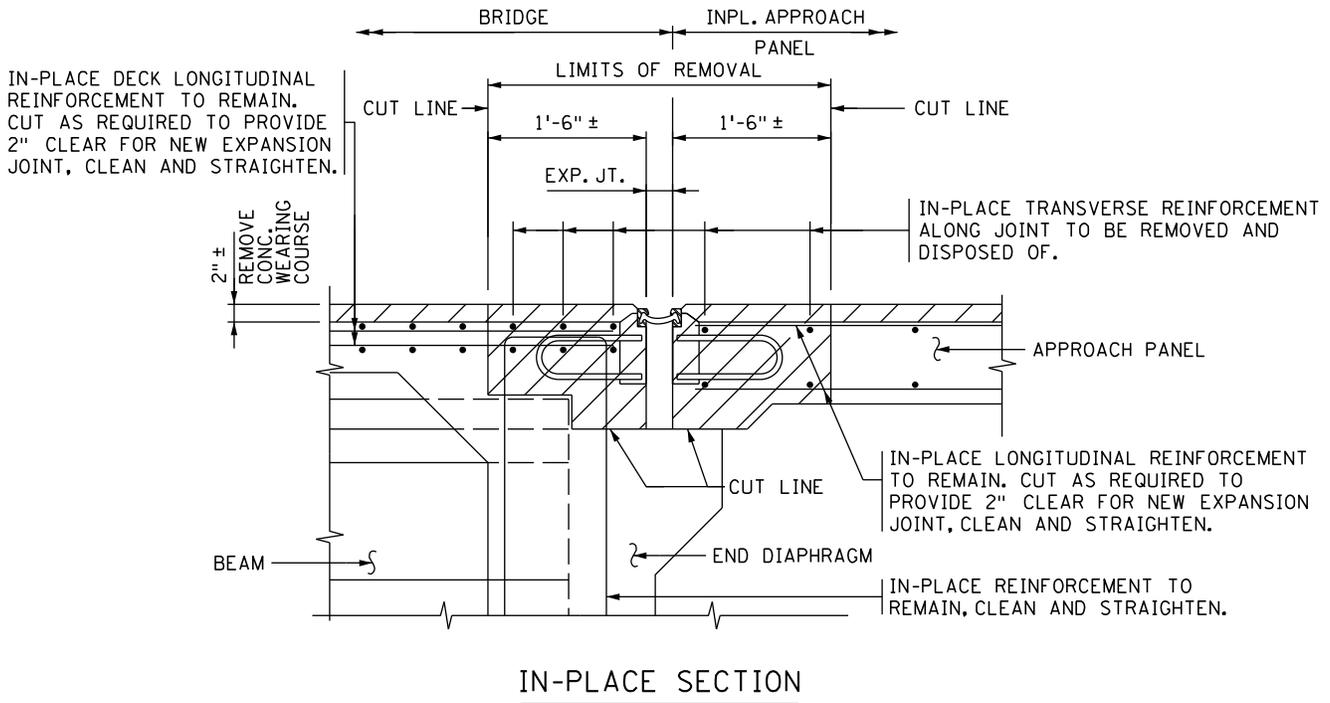
Use when replacing an in-place joint at a pier with a new fixed joint. See Figure 2.5.1.3.11. For payment, use item no.:

2433.603 "RECONSTRUCT FIXED JOINT TYPE B", LIN. FT.



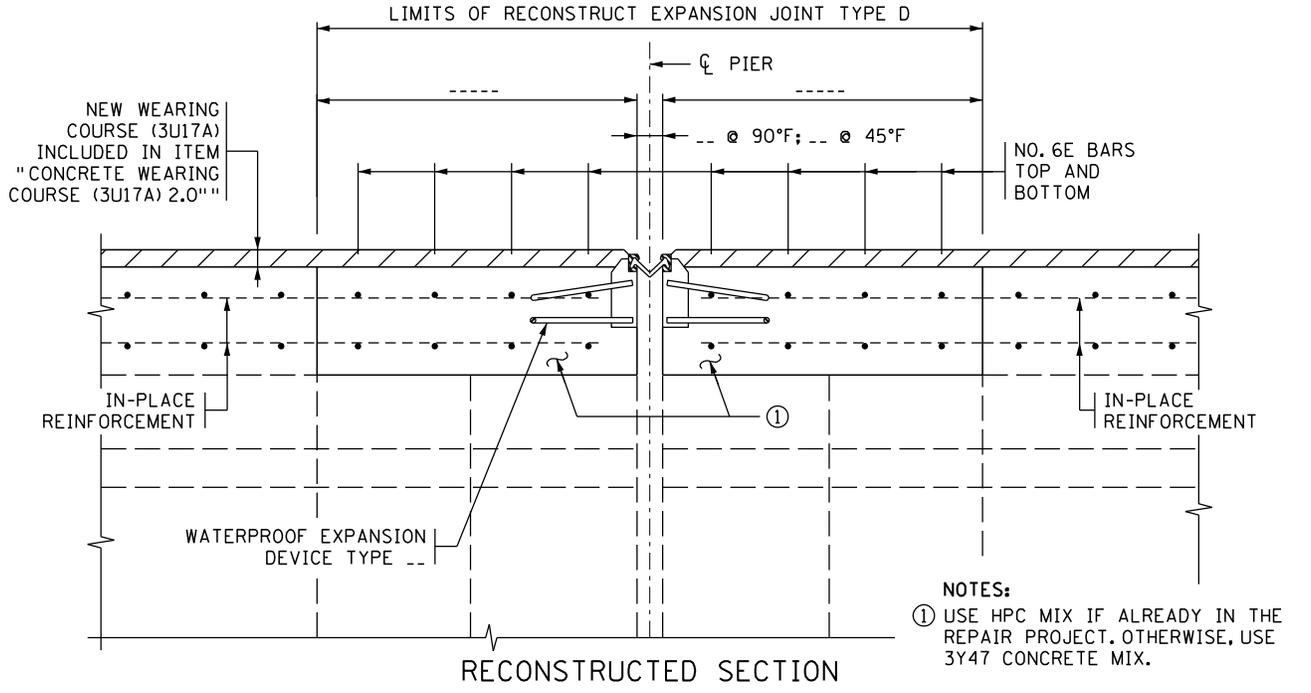
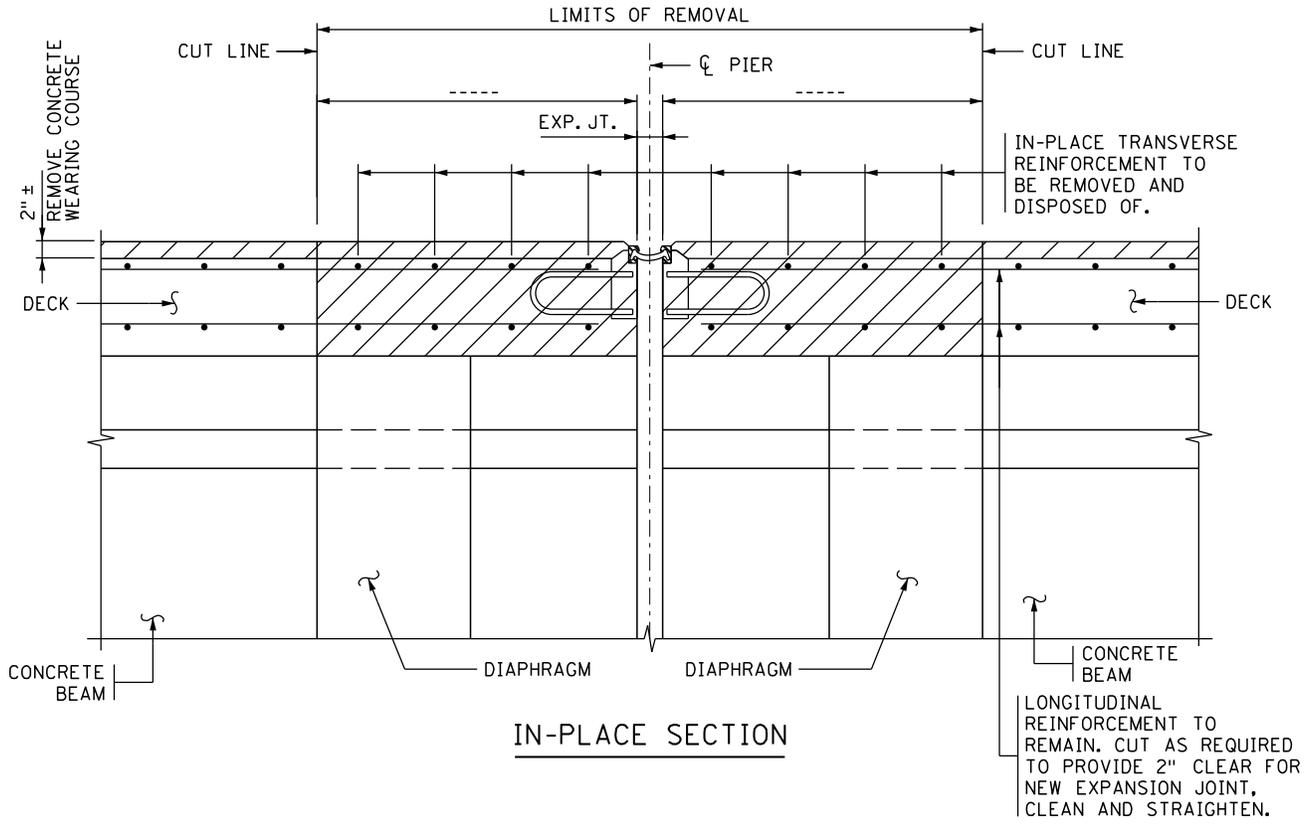
**RECONSTRUCT EXPANSION JOINT TYPE A**

**Figure 2.5.1.3.1  
Expansion Joints**



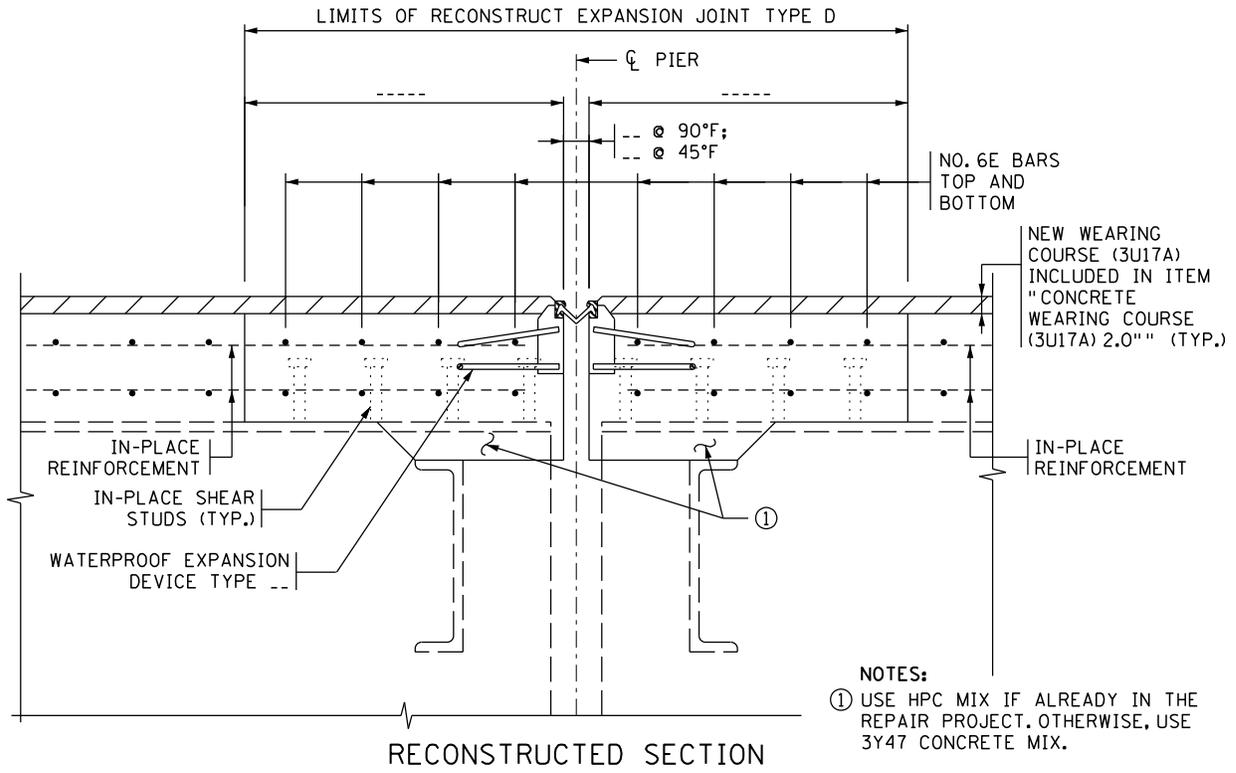
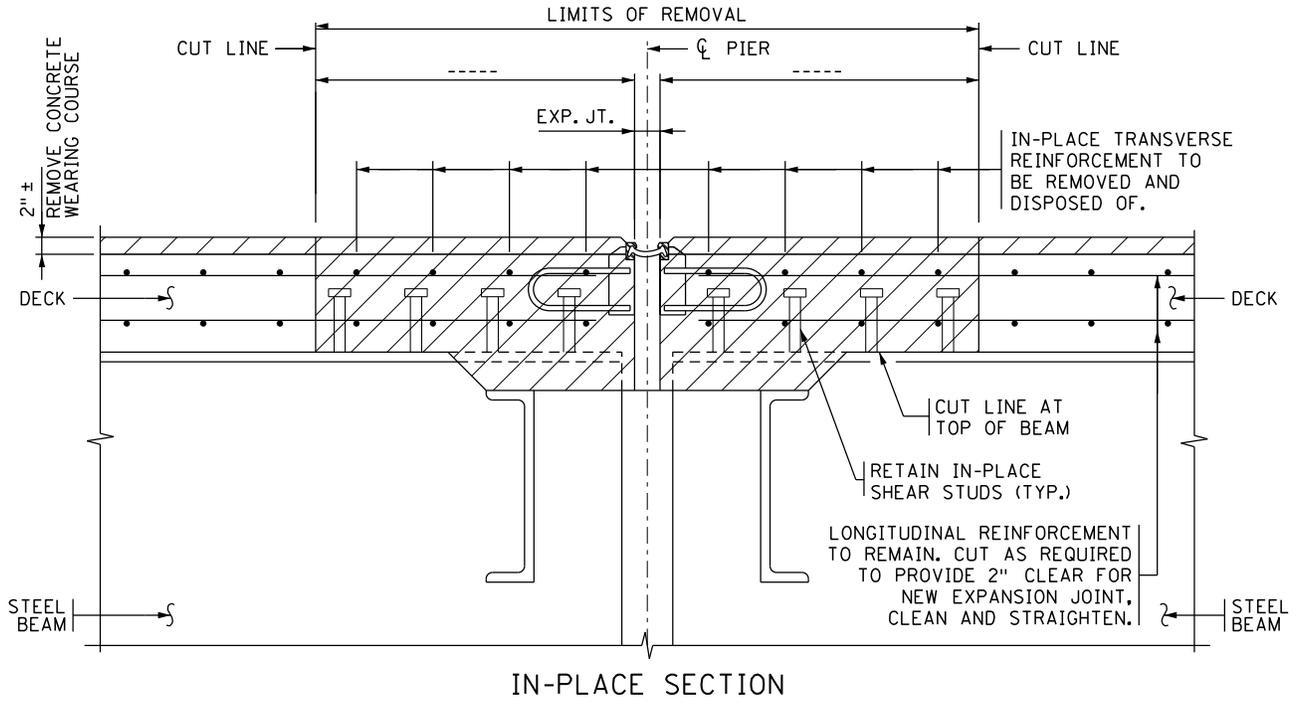
**RECONSTRUCT EXPANSION JOINT TYPE B**

**Figure 2.5.1.3.2  
Expansion Joints**



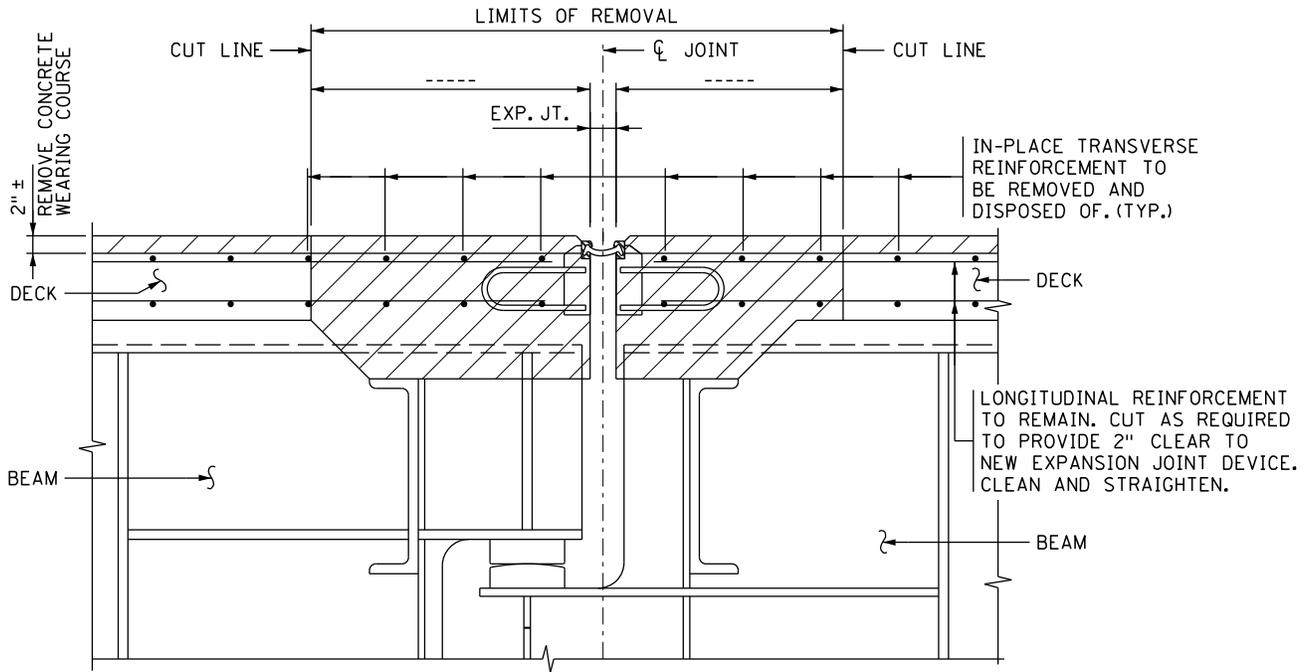
RECONSTRUCT EXPANSION JOINT TYPE D FOR CONCRETE BEAM SUPERSTRUCTURE

**Figure 2.5.1.3.3  
Expansion Joints**

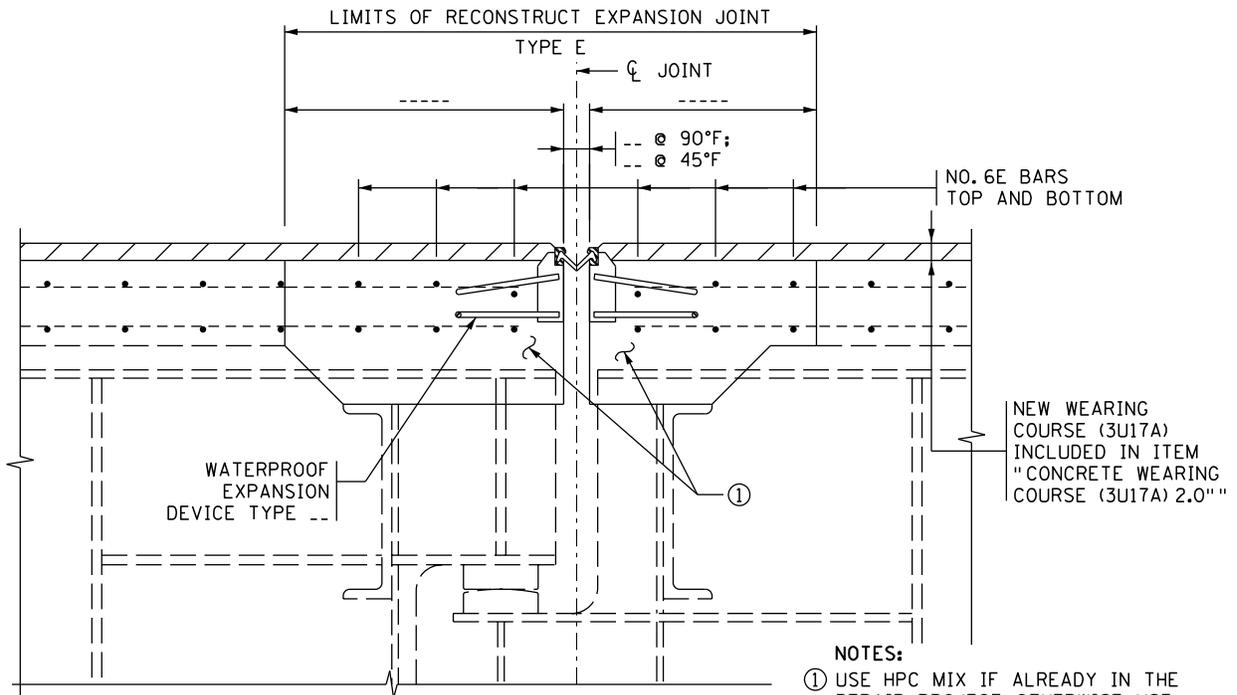


RECONSTRUCT EXPANSION JOINT TYPE D FOR STEEL BEAM SUPERSTRUCTURE

**Figure 2.5.1.3.4**  
**Expansion Joints**



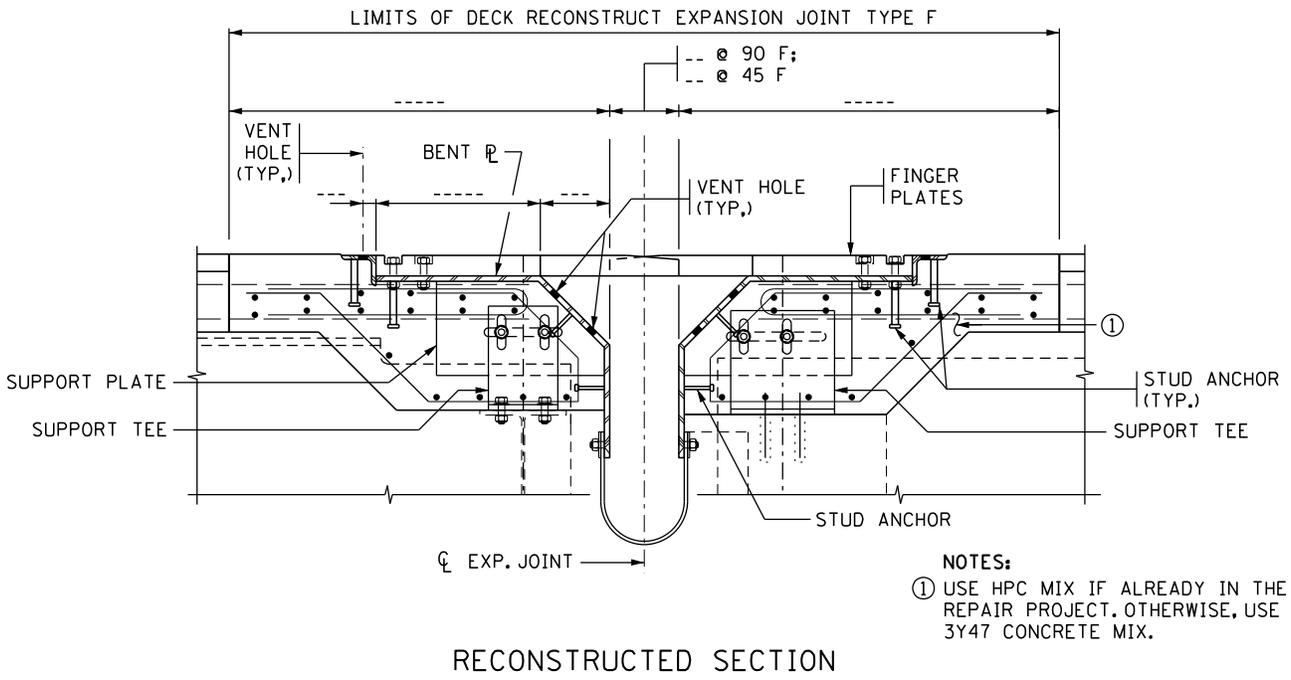
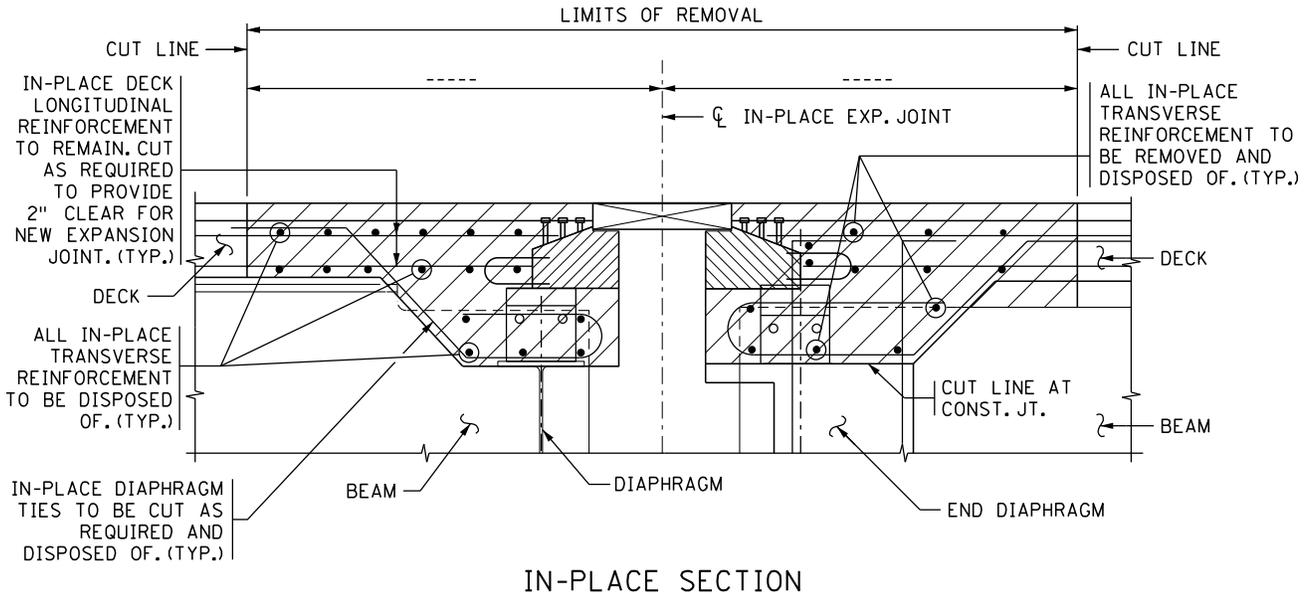
IN-PLACE SECTION



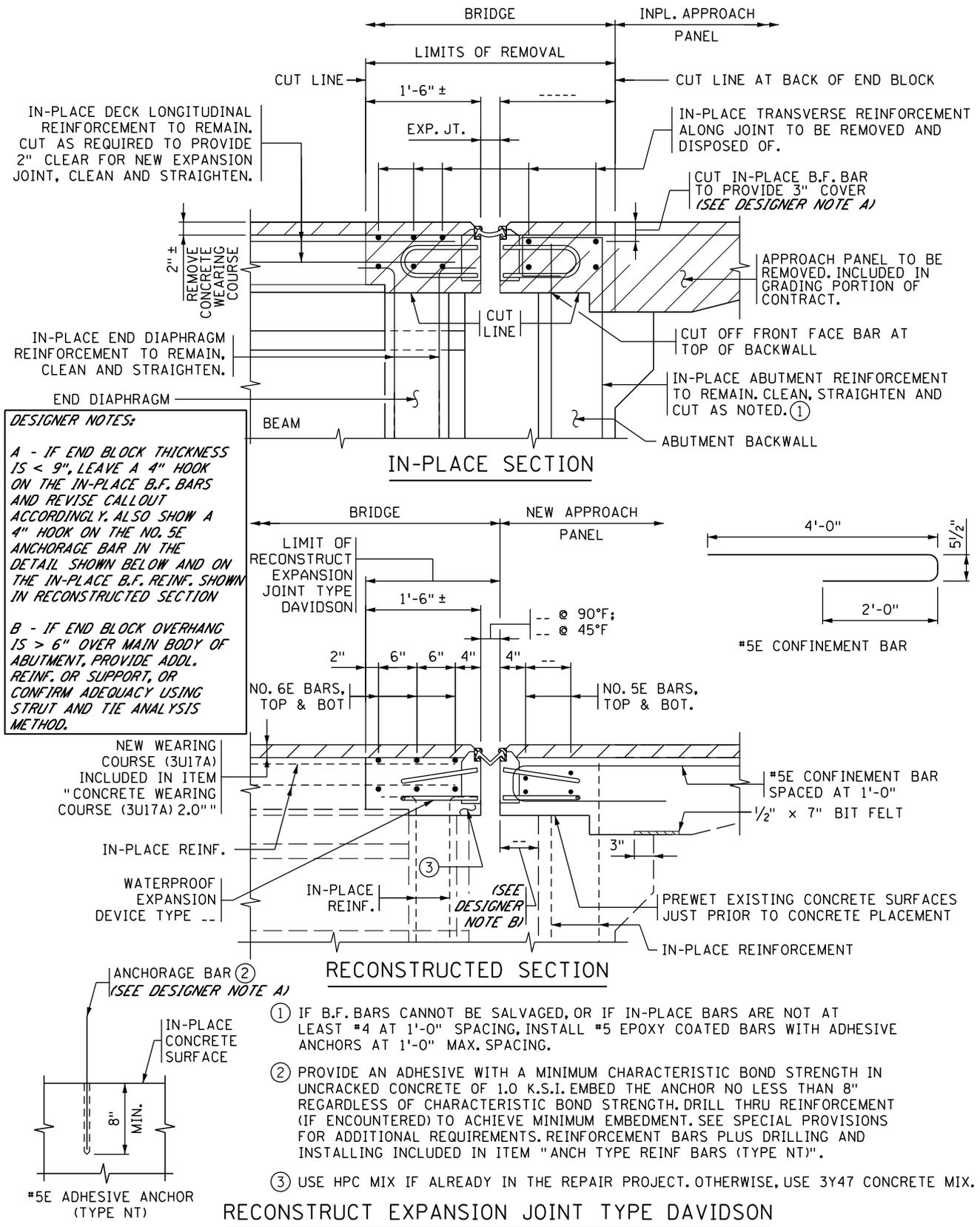
RECONSTRUCTED SECTION

RECONSTRUCT EXPANSION JOINT TYPE E

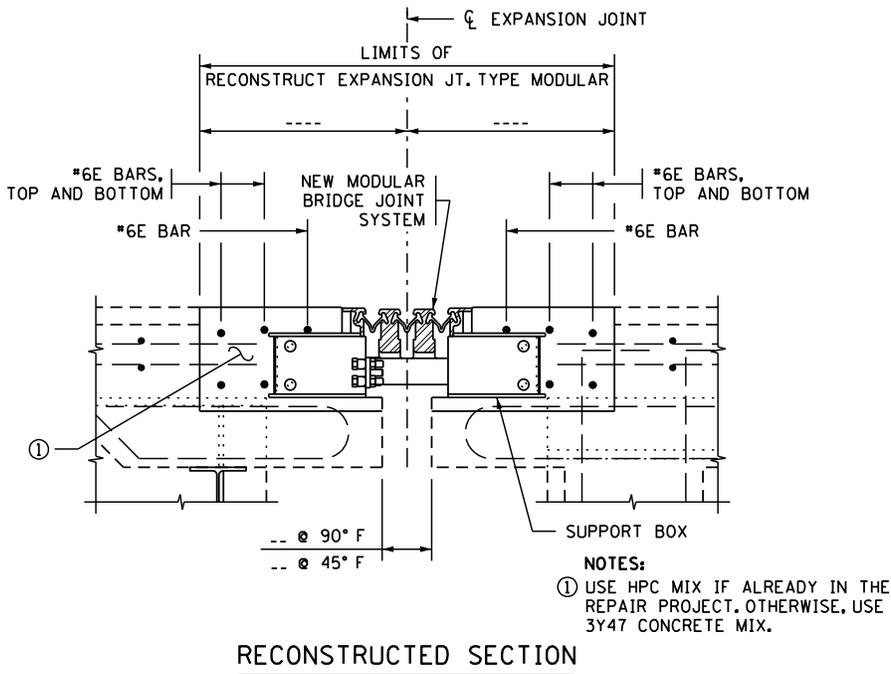
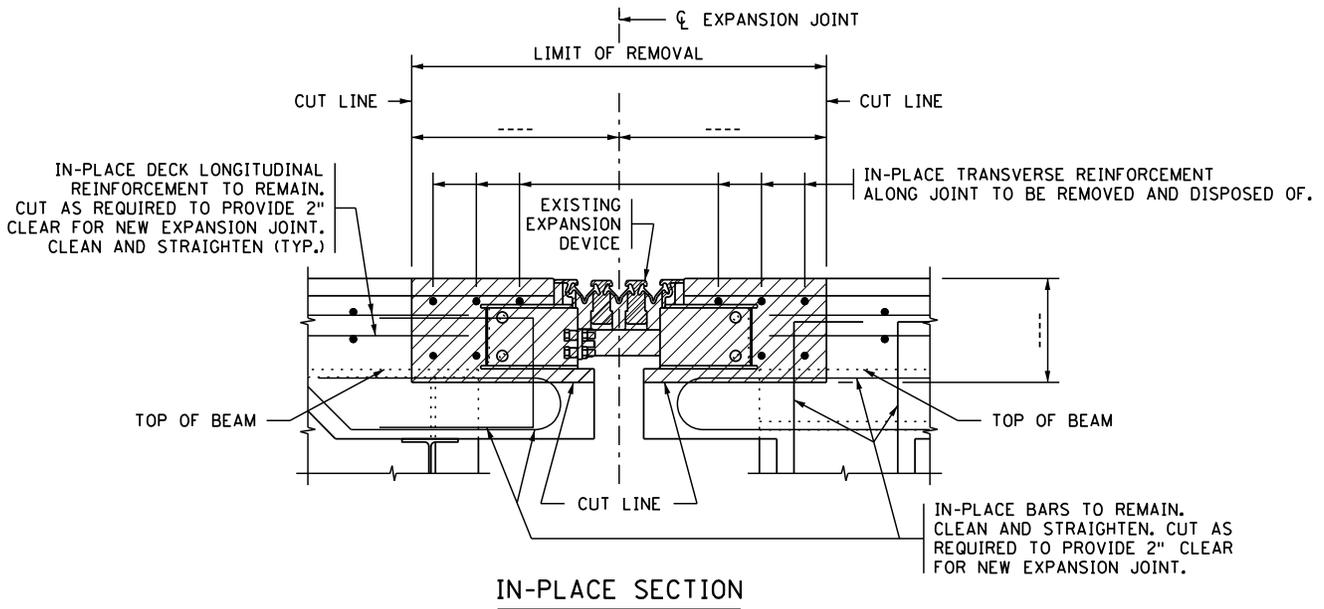
**Figure 2.5.1.3.5  
Expansion Joints**



**Figure 2.5.1.3.6**  
**Expansion Joints**

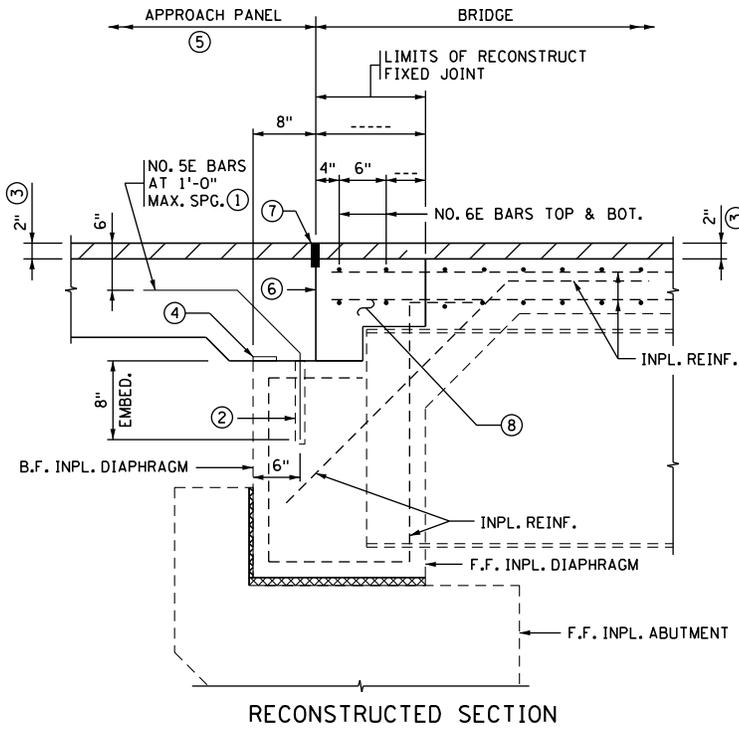
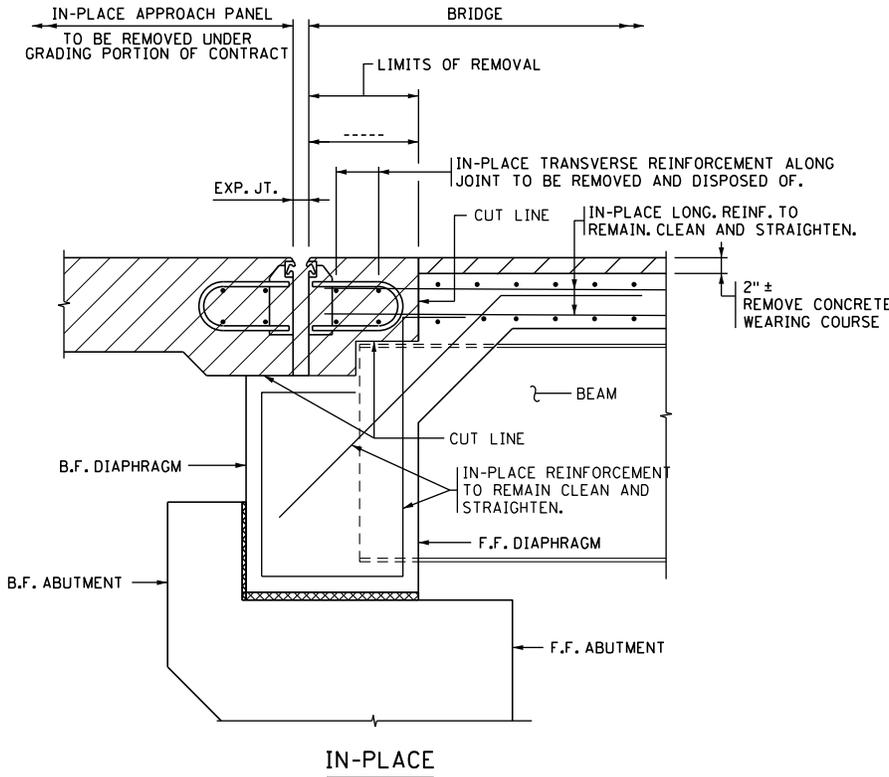


**Figure 2.5.1.3.7**  
**Expansion Joints**



RECONSTRUCT EXPANSION JOINT TYPE MODULAR

**Figure 2.5.1.3.8**  
**Expansion Joints**



**NOTES:**

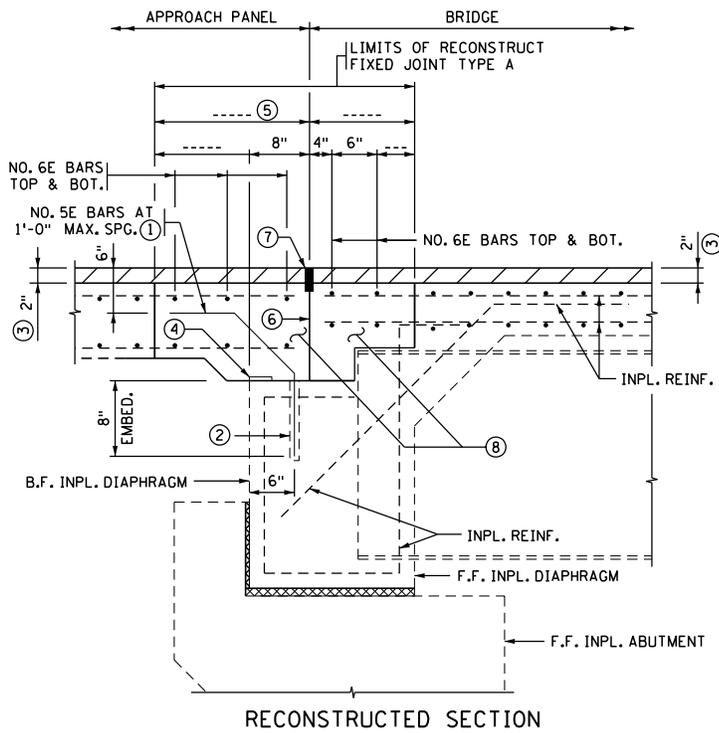
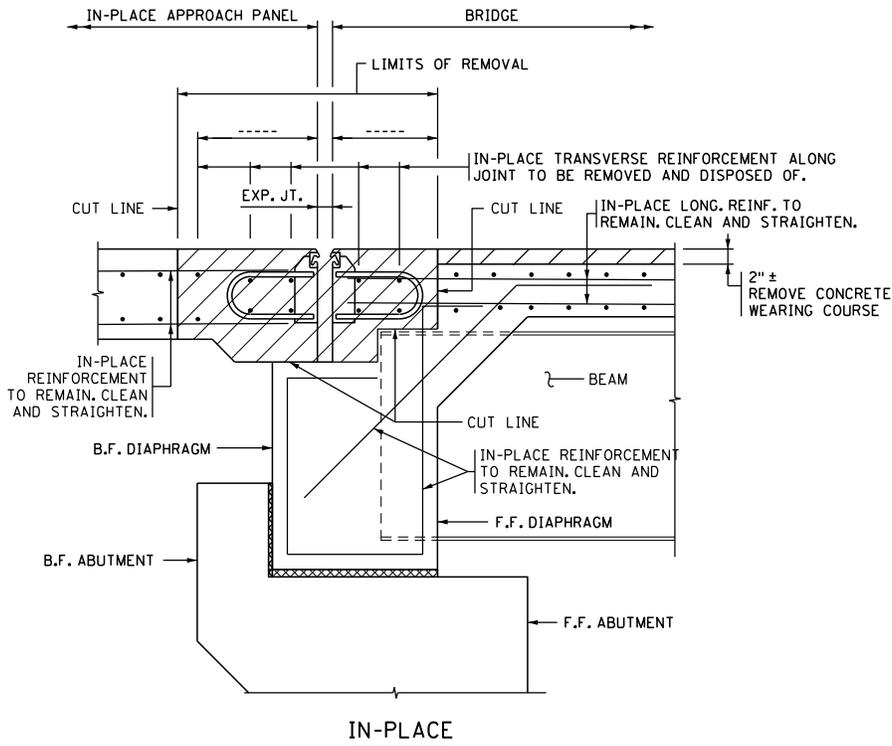
F.F. DENOTES FRONT FACE.

B.F. DENOTES BACK FACE.

- ① TIE NEW APPROACH PANEL TO CONCRETE END DIAPHRAGM. REINFORCEMENT BARS PLUS DRILLING AND INSTALLING INCLUDED IN ITEM "ANCH TYPE REINF BARS (TYPE NT)".
- ② PROVIDE AN ADHESIVE WITH A MINIMUM CHARACTERISTIC BOND STRENGTH IN UNCRACKED CONCRETE OF 1.0 K.S.I. EMBED THE ANCHOR NO LESS THAN 8" REGARDLESS OF CHARACTERISTIC BOND STRENGTH. DRILL THROUGH REINFORCEMENT (IF ENCOUNTERED) TO ACHIEVE MINIMUM EMBEDMENT. SEE SPECIAL PROVISIONS FOR ADDITIONAL REQUIREMENTS.
- ③ NEW CONCRETE WEARING COURSE (3U17A). INCLUDED IN ITEM "CONCRETE WEARING COURSE (3U17A) 2.0"
- ④ 1/2" X 3" BIT FELT. INCLUDED IN ITEM "RECONSTRUCT FIXED JOINT".
- ⑤ APPROACH PANEL TO BE CONSTRUCTED UNDER GRADING PORTION OF CONTRACT. DELAY PLACEMENT OF APPROACH PANEL MINIMUM OF 24 HOURS AFTER PLACING CONCRETE FOR "RECONSTRUCT FIXED JOINT".
- ⑥ DO NOT APPLY BONDING AGENT AT THIS CONSTRUCTION JOINT.
- ⑦ C2H CONTRACTION JOINT SEE GRADING PLANS.
- ⑧ USE HPC MIX IF ALREADY IN THE REPAIR PROJECT. OTHERWISE, USE 3Y47 CONCRETE MIX.

RECONSTRUCT FIXED JOINT

**Figure 2.5.1.3.9**  
**Fixed Joints**



**NOTES:**

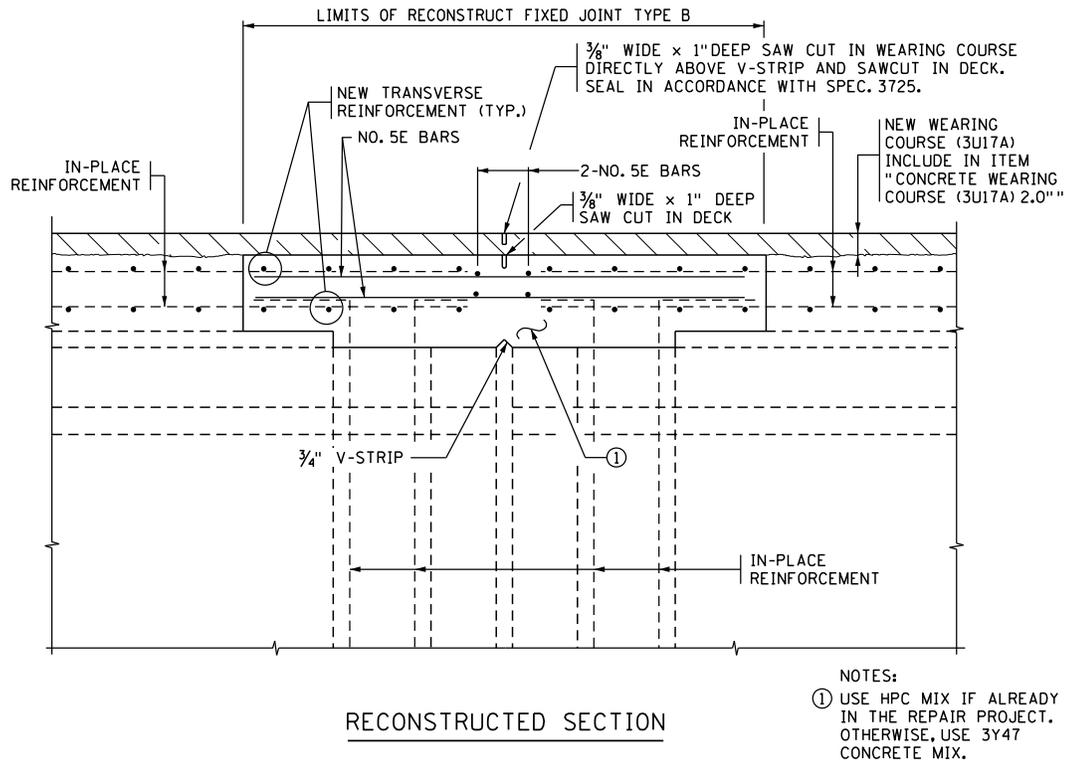
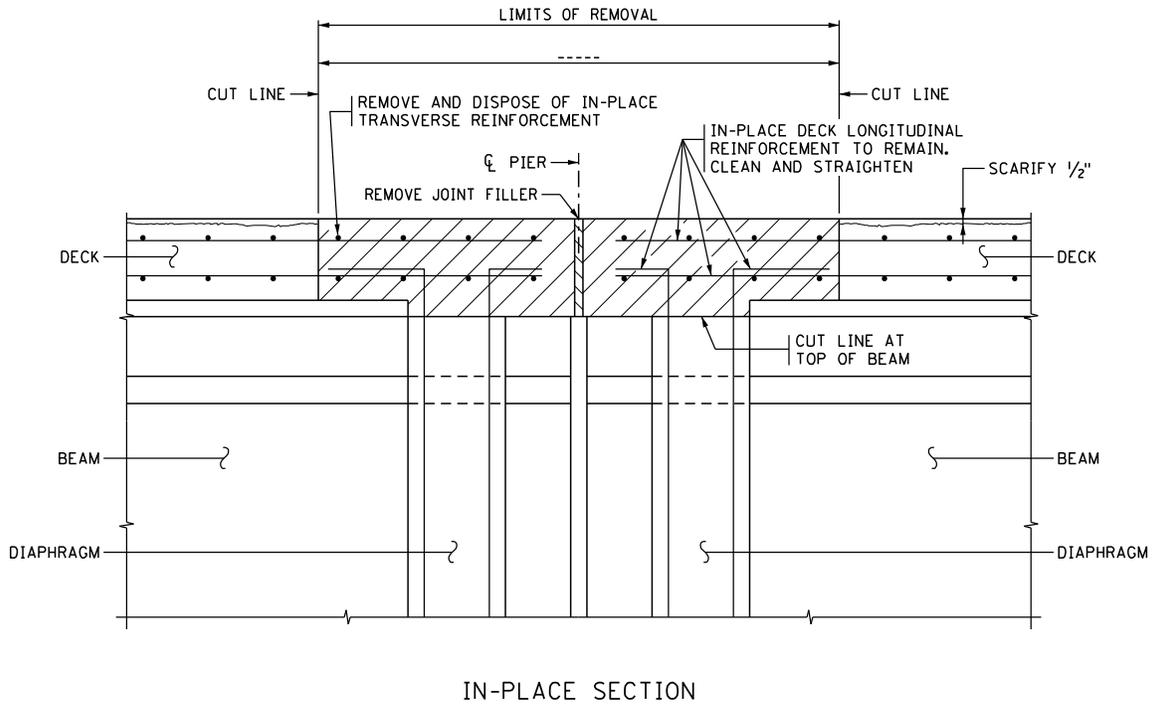
F.F. DENOTES FRONT FACE.

B.F. DENOTES BACK FACE.

- ① TIE APPROACH PANEL TO CONCRETE END DIAPHRAGM, REINFORCEMENT BARS PLUS DRILLING AND INSTALLING INCLUDED IN ITEM "ANCH TYPE REINF BARS (TYPE NT)".
- ② PROVIDE AN ADHESIVE WITH A MINIMUM CHARACTERISTIC BOND STRENGTH IN UNCRACKED CONCRETE OF 1.0 K.S.I. EMBED THE ANCHOR NO LESS THAN 8" REGARDLESS OF CHARACTERISTIC BOND STRENGTH, DRILL THROUGH REINFORCEMENT (IF ENCOUNTERED) TO ACHIEVE MINIMUM EMBEDMENT, SEE SPECIAL PROVISIONS FOR ADDITIONAL REQUIREMENTS.
- ③ NEW CONCRETE WEARING COURSE (3U17A), INCLUDED IN ITEM "CONCRETE WEARING COURSE (3U17A) 2.0"
- ④ 1/2" X 3" BIT FELT, INCLUDED IN ITEM "RECONSTRUCT FIXED JOINT".
- ⑤ DELAY PLACEMENT OF CONCRETE ON APPROACH PANEL SIDE OF FIXED JOINT A MINIMUM OF 24 HOURS AFTER PLACEMENT OF CONCRETE ON BRIDGE SIDE OF FIXED JOINT.
- ⑥ DO NOT APPLY BONDING AGENT AT THIS CONSTRUCTION JOINT.
- ⑦ C2H CONTRACTION JOINT SEE GRADING PLANS.
- ⑧ USE HPC MIX IF ALREADY IN THE REPAIR PROJECT. OTHERWISE, USE 3Y47 CONCRETE MIX.

**RECONSTRUCT FIXED JOINT TYPE A**

**Figure 2.5.1.3.10**  
**Fixed Joints**



**RECONSTRUCT FIXED JOINT TYPE B**

**Figure 2.5.1.3.11**  
**Fixed Joints**

**2.5.2 Substructure** [Future manual content]

**2.5.2.1 Abutments** The following figures show typical details for the repair of abutment paving brackets:

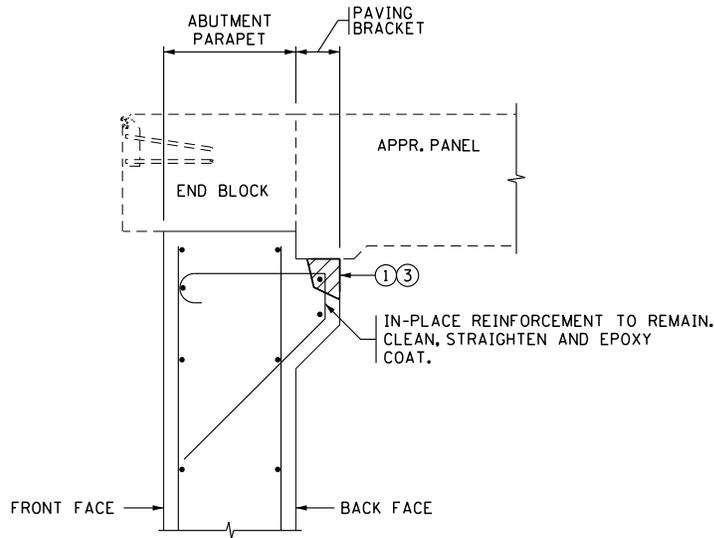
Figure 2.5.2.1.1 Repair Paving Bracket

Figure 2.5.2.1.2 Reconstruct Paving Bracket

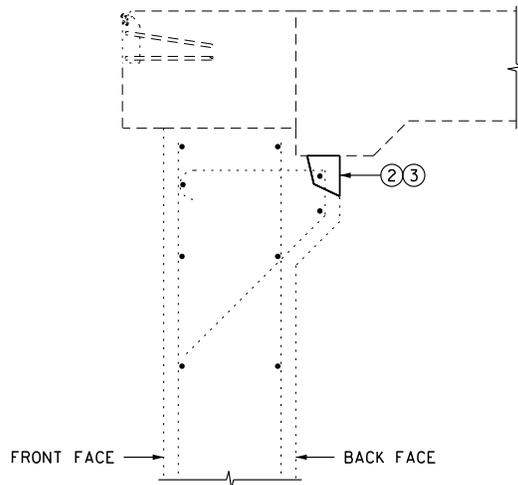
Figure 2.5.2.1.3 Reconstruct Paving Bracket and Wall

**Paving Bracket Repair Pay Items**

- Item No. 2433.603 "Repair Paving Bracket", Lin. Ft.
- Item No. 2433.603 "Reconstruct Paving Bracket", Lin. Ft.
- Item No. 2433.618 "Reconstruct Paving Bracket and Wall", Sq. Ft.



IN-PLACE SECTION REMOVAL

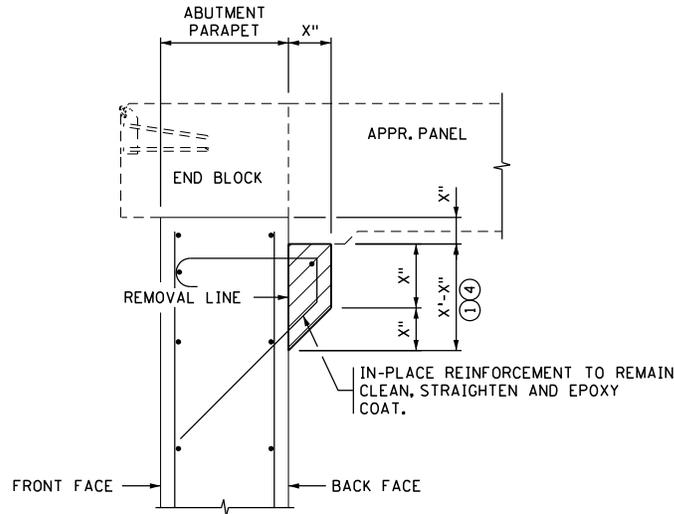


REPAIRED SECTION

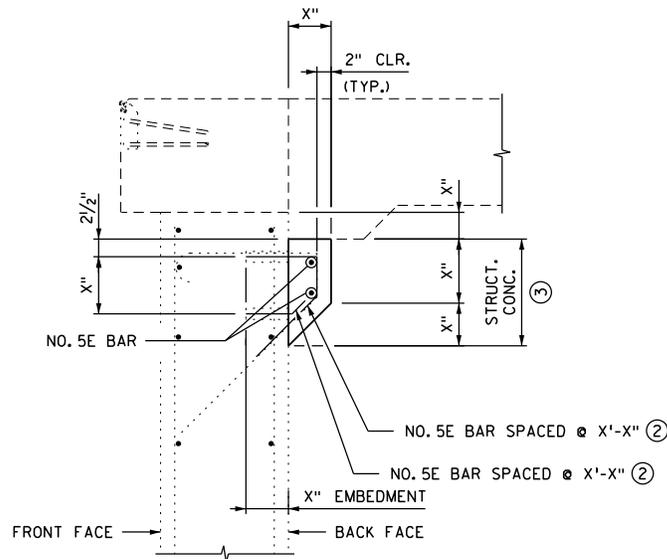
- ① REMOVE ANY DETERIORATED AREAS OF PAVING BRACKET TO SOUND CONCRETE IN ACCORDANCE WITH THE SPECIAL PROVISIONS.
- ② REPLACE REMOVED CONCRETE WITH CONCRETE MIX NO. 3B52. COAT SURFACE OF EXISTING CONCRETE WITH BONDING GROUT PER SPECIAL PROVISIONS PRIOR TO CASTING.
- ③ INCLUDED IN ITEM "REPAIR PAVING BRACKET".

REPAIR PAVING BRACKET

**Figure 2.5.2.1.1**  
**Paving Brackets**



IN-PLACE SECTION REMOVAL



RECONSTRUCTED SECTION (4)

- ① REMOVE PAVING BRACKET IN ITS ENTIRETY TO SOUND CONCRETE IN ACCORDANCE WITH THE SPECIAL PROVISIONS.
- ② SUPPLEMENT IN-PLACE REINFORCEMENT AS NEEDED WITH NEW BARS AND ADHESIVE ANCHORS. PROVIDE AN ADHESIVE ANCHOR WITH A MINIMUM CHARACTERISTIC BOND STRENGTH IN UNCRACKED CONCRETE OF 1.0 K.S.I. EMBED THE ANCHORAGE NO LESS THAN 6" INTO SOLID CONCRETE. INCLUDED IN ITEM "ANCH TYPE REINF BARS (TYPE L)".
- ③ REPLACE CONCRETE WITH CONCRETE MIX NO. 3B52. COAT SURFACE OF EXISTING CONCRETE WITH BONDING GROUT PER SPECIAL PROVISIONS PRIOR TO CASTING.
- ④ INCLUDED IN ITEM "RECONSTRUCT PAVING BRACKET" EXCEPT AS NOTED.

RECONSTRUCT PAVING BRACKET

**Figure 2.5.2.1.2**  
**Paving Brackets**



**2.5.2.2 Piers**

[Future manual content]

**2.6 Construction Requirements**

MnDOT's general practices and guidelines for the construction of bridges are presented in MnDOT's *Bridge Construction Manual*, which can be accessed from the MnDOT Bridge Office web site.

Provide the required submittals and the qualifications of the individuals responsible for the preparation of falsework and other submittals in the contract documents for the project.

Falsework and forms are to be designed in accordance with the AASHTO *Guide Design Specifications for Bridge Temporary Works*. Falsework submittals must meet the requirements of Bridge Special Provision No. SB2018-2401.2.

Submittals describing proposed temporary shoring for works adjacent to railroad tracks require approval by the railroad.

Provide details of temporary shoring in the plans with consideration of the domestic availability of the sheeting materials. Frequently, showing the location of the sheeting and the minimum required section modulus is sufficient. However, designers should satisfy themselves that adequate clearances have been provided for at least one reasonable shoring scheme for staged construction projects. If more complex details are required, they must be provided in the plans. See Article 11.3.7 of this manual for more guidance.

**APPENDIX 2-A  
BRIDGE TYPE NUMBERS**

**MINNESOTA BRIDGE TYPE IDENTIFICATION NUMBER (3 characters)**

<b>First Digit (Superstructure Material)</b>	<b>Second &amp; Third Digits (Bridge Type)</b>
1 Cast-In-Place Concrete	01 Beam Span
2 Cast-In-Place Concrete Continuous	02 Low Truss
3 Steel	03 High Truss
4 Steel Continuous	04 Deck Truss
5 Prestressed Pretensioned or Precast Concrete	05 Thru Girder
6 Prestressed Pretensioned Continuous Concrete	06 Deck Girder
7 Timber	07 Box Girder
8 Masonry	08 Rigid Frame
9 Wrought or Cast Iron	09 Slab Span
O Other	10 Slab Span-Voided
A Aluminum	11 Channel Span
P Prestressed Post-Tensioned	12 Arch
	13 Box Culvert
	14 Pipe Culvert (Round)
	15 Pipe Arch
	16 Long Span
	17 Tunnel
	18 Movable
	19 Other
	20 Double Tee
	21 Quad Tee
	22 Bulb Tee
	23 Suspension
	24 Tied Arch
	25 Cable Stay
	26 Inverted T

<b>EXAMPLES</b>	
<b>BRIDGE TYPE</b>	<b>ID NUMBER</b>
Precast Concrete Box Culvert	513
Simple Span Cast-In-Place Concrete Slab	109
Tunnel in Rock	017
Pretensioned Concrete Beam Span	501 approach span
Steel Continuous Beam Span	401 main span
Post-Tensioned Concrete Box Girder	P07

Note: A bridge may have different identification numbers for each span. Identify all span types accordingly.

**APPENDIX 2-B  
STANDARD ABBREVIATIONS**

**A**

AASHTO ..... American Association of State Highway and Transportation Officials  
 ABT. .... About  
 ABUT. .... Abutment  
 AADT ..... Annual Average Daily Traffic  
 ADTT ..... Average Daily Truck Traffic  
 ALT. .... Alternate  
 APPR. .... Approach  
 APPROX. .... Approximate (or Approximately)  
 ASSY. .... Assembly  
 AZ. .... Azimuth  
 @ ..... At

**B**

B.F. .... Back Face  
 BIT. .... Bituminous  
 B.M. .... Bench Mark  
 BM ..... Beam  
 BOT. .... Bottom  
 BR. .... Bridge  
 BRG. .... Bearing  
 BTWN. .... Between

**C**

C & G..... Curb and Gutter  
 C-I-P ..... Cast-In-Place  
 CL ..... Centerline  
 CL. (or CLR.)..... Clear  
 C.M.P. .... Corrugated Metal Pipe  
 COL. .... Column  
 COMP. .... Composite  
 CONC. .... Concrete  
 CONST. .... Construction  
 CONT. .... Continuous (or Continued)  
 C.S.A.H. .... County State Aid Highway  
 CU. .... Cubic  
 CULV. .... Culvert

**D**

D.C. .... Degree of Curve  
 DET. .... Detail  
 D.H.V. .... Design Hourly Volume  
 D.H.W. .... Design High Water  
 DIA. .... Diameter  
 DIAPH. .... Diaphragm  
 DL ..... Dead Load  
 DWL. .... Dowel

**E**

E. .... East  
 E.B.L. .... East Bound Lane(s)  
 E.F. .... Each Face  
 EA. .... Each  
 ELEV. (or EL.) ..... Elevation  
 EMBED. .... Embedment  
 ENGR. .... Engineer  
 EQ. .... Equal  
 EXP. .... Expansion

**F**

F. .... Fahrenheit  
 F.B.M. .... Foot Board Measure  
 F.F. .... Front Face  
 F.L. .... Flowline  
 FIN. .... Finished  
 FIX ..... Fixed  
 FT. .... Foot (or Feet)  
 FTG. .... Footing

**G**

G1 ..... Grade One  
 G2 ..... Grade Two  
 GA. .... Gage

**APPENDIX 2-B (Continued)  
STANDARD ABBREVIATIONS**

**H**

HCAADT ..... Heavy Commercial Annual  
Average Daily Traffic  
H.W. .... High Water  
HORIZ. .... Horizontal  
HWY. .... Highway

**I**

INPL. .... In-place  
I.D. .... Inside Diameter

**J**

JCT. .... Junction  
JT. .... Joint

**K**

KWY. .... Keyway

**L**

L. .... Length of Curve  
LL ..... Live Load  
L.W. .... Low Water  
LB. .... Pound  
LIN. .... Linear  
LT. .... Left  
LONG. (or LONGIT.)..... Longitudinal

**M**

m ..... Meter  
mm ..... Millimeter  
M.B.M. .... Thousand Board Feet  
M.L. .... Main Line  
M.O. .... Maximum Offset  
MAX. .... Maximum  
MIN. .... Minimum  
MISC. .... Miscellaneous

**N**

N. (or NO.)..... North  
N.B.L. .... North Bound Lane(s)  
NO. .... Number

**O**

O.D. .... Outside Diameter

**P**

P.C. .... Point of curvature  
P.C.C. .... Point of compound Curve  
P.G. .... Profile Grade  
P.I. .... Point of Intersection  
P.O.C. .... Point on Curve  
P.O.T. .... Point on Tangent  
P.S.I. .... Pounds per Square Inch  
P.T. .... Point of Tangency  
PED. .... Pedestrian  
PL ..... Plate  
PRESTR. .... Prestressed  
PROJ. .... Project (or Projection)  
PROV. .... Provision  
PT. .... Point

**R**

R. .... Radius  
R.O.W. .... Right of Way  
R.R. .... Railroad  
R.S.C. .... Rigid Steel Conduit  
RDWY. .... Roadway  
REINF. .... Reinforced (or Reinforcing/ment)  
REQ'D..... Required  
REV. .... Revised  
RT. .... Right

**S**

S. (or SO.) ..... South  
S.B.L. .... South Bound Lane(s)  
SEC. .... Section  
SDWK. .... Sidewalk  
SHLDR. .... Shoulder

**APPENDIX 2-B (Continued)  
STANDARD ABBREVIATIONS**

**S (cont.)**

SHT. ....Sheet  
 SP. (or SPS.) .... Spaces  
 SPA. .... Spaced  
 SPEC. .... Special (or Specification)  
 SPG. ....Spacing  
 SQ. ....Square  
 STA. ....Station  
 STD. .... Standard  
 STIFF. ....Stiffener  
 STL. .... Steel  
 STR. (or STRUC.) ....Structure  
 SUBGR. .... Subgrade  
 SUPER. .... Superlevation  
 SUPERST. .... Superstructure  
 SYM. .... Symmetrical

**T**

T & B ..... Top and Bottom  
 T.H. ....Trunk Highway  
 T.T.C. ....Tangent to Curve  
 TAN. .... Tangent  
 TWP. ....Township  
 TYP. .... Typical

**V**

V.C. .... Vertical Curve  
 V.P.C. .... Vertical Point of Curvature  
 V.P.I. .... Vertical Point of Intersection  
 V.P.T. ....Vertical Point of Tangency  
 VAR. .... Varies  
 VERT. .... Vertical

**W**

W. ....West  
 W.B.L. ....West Bound Lane(s)  
 W.C. ....Wearing Course  
 W.P. .... Working Point  
 W.W. .... Wingwall

**Y**

YD. .... Yard

**APPENDIX 2-C  
STANDARD PLAN NOTES**

Use standard notes that are relevant to the project. Text found in brackets [ ] next to a standard note provides guidance on its use and should not be included in the Plan.

**A. DESIGN DATA AND PROJECTED TRAFFIC VOLUMES**

**a. NEW BRIDGES OTHER THAN CONCRETE BOX CULVERTS**

DESIGNED IN ACCORDANCE WITH 20\_\_ AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS

HL-93 LIVE LOAD

DEAD LOAD INCLUDES 20 POUNDS PER SQUARE FOOT ALLOWANCE FOR FUTURE WEARING COURSE MODIFICATIONS

[Use on all roadway bridge projects. Insert current year of specifications.]

DESIGNED IN ACCORDANCE WITH 20\_\_ AND CURRENT INTERIM AASHTO LRFD GUIDE SPECIFICATIONS FOR THE DESIGN OF PEDESTRIAN BRIDGES

PEDESTRIAN LIVE LOAD = 0.090 KSF

\_\_\*\_\_ TRUCK VEHICULAR LIVE LOAD

[Use on all pedestrian bridge projects. Insert current year of specifications.]

[\* Insert "H-5" for deck widths between parapet/curb faces  $\leq$  10 ft. For deck widths between parapet/curb faces  $>$  10 ft., insert "H-10"]

**MATERIAL DESIGN PROPERTIES:**

**REINFORCED CONCRETE:**

$f'_c = 4$  KSI CONCRETE

$f_y = 60$  KSI PLAIN AND EPOXY COATED BARS

$f_y = 75$  KSI STAINLESS STEEL BARS

$n = 7.3$  FOR REINFORCEMENT BARS

[Use on all projects. Delete stainless steel bars when not included in bridge plan.]

**PRETENSIONED CONCRETE:**

$f'_c =$  \_\_ KSI CONCRETE

$f_{pu} = 300$  KSI LOW RELAXATION STRANDS

$n = 1$  FOR PRETENSIONING STRANDS

\_\_\_\_  $f_{pu}$  FOR INITIAL PRESTRESS

[Use on bridges with pretensioned beams. Insert  $f'_c$  used on beam detail sheet, rounded to 0.1 ksi. Insert fraction of  $f_{pu}$  used for initial prestress on beam detail sheet.]

**APPENDIX 2-C (Continued)**  
**STANDARD PLAN NOTES**

Use standard notes that are relevant to the project. Text found in brackets [ ] next to a standard note provides guidance on its use and should not be included in the Plan.

**A. DESIGN DATA AND PROJECTED TRAFFIC VOLUMES (CONT'D)**

**a. NEW BRIDGES OTHER THAN CONCRETE BOX CULVERTS (CONT'D)**

POST-TENSIONED CONCRETE:

$f'_c = \underline{\hspace{1cm}}$  KSI CONCRETE

$f_{pu} = 270$  KSI LOW RELAXATION STRANDS

$n = 1$  FOR POST-TENSIONING STRANDS

$0.75 f_{pu}$  FOR INITIAL PRESTRESS AFTER ANCHOR SET

TOP OF DECK DESIGNED FOR ZERO TENSION UNDER SERVICE LOADS

[Use as required, e.g. post-tensioned slabs or box girders. Insert concrete strength.]

STRUCTURAL STEEL:

$F_y = 36$  KSI STRUCTURAL STEEL SPEC. 3306 (PAINTED)

$F_y = 50$  KSI STRUCTURAL STEEL SPEC. 3309 (PAINTED)

$F_y = 70$  KSI STRUCTURAL STEEL SPEC. 3317 (HIGH PERFORMANCE)  
(PAINTED)

[Use as required on bridges with steel components. Include description within parentheses as needed.]

WOOD:

$F_{bo} = \underline{\hspace{1cm}}$  KSI PILE CAPS

$F_{bo} = \underline{\hspace{1cm}}$  KSI SAWN STRINGERS AND TIMBER RAILS

$F_{bo} = \underline{\hspace{1cm}}$  KSI GLUED LAMINATED TIMBER RAILS

$F_{bo} = \underline{\hspace{1cm}}$  KSI GLUED LAMINATED STRINGERS

$F_{bo} = \underline{\hspace{1cm}}$  KSI GLUED LAMINATED DECK PANELS

$F_{bo} = \underline{\hspace{1cm}}$  KSI NAIL LAMINATED DECK PANELS

$F_{bo} = \underline{\hspace{1cm}}$  KSI RAIL POSTS

$F_{bo} = \underline{\hspace{1cm}}$  KSI ALL OTHER WOOD

[Use as required on bridges with wood components. Insert reference design values.]

DESIGN SPEED:

OVER =            MPH

[Use on all projects. Insert speed.]

UNDER =            MPH

[Use as required. Insert speed.]

**APPENDIX 2-C (Continued)**  
**STANDARD PLAN NOTES**

Use standard notes that are relevant to the project. Text found in brackets [ ] next to a standard note provides guidance on its use and should not be included in the Plan.

**A. DESIGN DATA AND PROJECTED TRAFFIC VOLUMES (CONT'D)**

**a. NEW BRIDGES OTHER THAN CONCRETE BOX CULVERTS (CONT'D)**

DECK AREA = \_\_\_\_\_ SQUARE FEET

[Use on all projects. Insert area based on dimensions from coping to coping and begin bridge to end bridge.]

20\_\_ PROJECTED TRAFFIC VOLUMES

ROADWAY OVER            ROADWAY UNDER

\_\_\_\_\_ AADT            \_\_\_\_\_

\_\_\_\_\_ DHV            \_\_\_\_\_

\_\_\_\_\_ HCAADT            \_\_\_\_\_

[Use when provided on the Preliminary Plan. Insert appropriate values in vehicles per day. Note that HCAADT is Heavy Commercial Annual Average Daily Traffic, which can be considered equivalent to the Average Daily Truck Traffic (ADTT) used by the AASHTO LRFD Bridge Design Specifications.]

HL-93 LRFR

BRIDGE OPERATING RATING FACTOR RF = \_\_\_\_\_

[Use on all projects. Insert rating factor.]

**b. PRECAST CONCRETE BOX CULVERTS**

DESIGNED IN ACCORDANCE WITH \_\_\_\_\_ AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS

[For standard precast concrete box culvert designs, insert date of specifications that standards are based on, which can be found at the top of Bridge Standard Plans Figure 5-395.100(A) Precast Concrete Box Culvert - Basis of Design. For non-standard designs, insert current year of specifications.]

HL-93 LIVE LOAD

BARREL SPAN = \_\_\_\_\_

BARREL RISE = \_\_\_\_\_

BARREL LENGTH = \_\_\_\_\_

EST. MIN. FILL DEPTH (A) = \_\_\_\_\_

EST. MAX. FILL DEPTH (B) = \_\_\_\_\_

SKEW ANGLE = \_\_\_\_ ° \_\_\_\_ ' \_\_\_\_ "

[Use on all precast concrete box culvert projects. Insert appropriate values. For estimated min. and max. fill depths, also show (A) and (B) in the box culvert elevation view to depict the min. and max. fill heights at the outside edge of the roadway shoulder.]

**APPENDIX 2-C (Continued)**  
**STANDARD PLAN NOTES**

Use standard notes that are relevant to the project. Text found in brackets [ ] next to a standard note provides guidance on its use and should not be included in the Plan.

**A. DESIGN DATA AND PROJECTED TRAFFIC VOLUMES (CONT'D)**

**b. PRECAST CONCRETE BOX CULVERTS (CONT'D)**

DESIGN SPEED = \_\_\_ MPH

CURRENT AADT (YEAR) = \_\_\_\_\_(20\_\_)

PROJECTED AADT (YEAR) = \_\_\_\_\_(20\_\_)

CURRENT HCAADT (YEAR) = \_\_\_\_\_(20\_\_)

[Use on all precast concrete box culvert projects. Insert appropriate values.]

MATERIAL DESIGN PROPERTIES:

PRECAST REINFORCED CONCRETE:

$f'_c$  = \_\_\_ KSI CONCRETE

$f_y$  = 65 KSI WELDED WIRE REINFORCEMENT

$f_y$  = 60 KSI REINFORCEMENT BARS

[Use on all precast concrete box culvert projects. Insert concrete strength.]

HL-93 LRFR

BRIDGE OPERATING RATING FACTOR RF = \_\_\_\_\_

[Use on all precast concrete box culvert projects where the box culvert has been assigned a bridge number. For standard designs, insert 1.3 rating factor. For non-standard designs, insert calculated rating factor.]

**APPENDIX 2-C (Continued)**  
**STANDARD PLAN NOTES**

Use standard notes that are relevant to the project. Text found in brackets [ ] next to a standard note provides guidance on its use and should not be included in the Plan.

**B. CONSTRUCTION NOTES**

THE 20\_\_ EDITION OF THE MINNESOTA DEPARTMENT OF TRANSPORTATION "STANDARD SPECIFICATIONS FOR CONSTRUCTION" SHALL GOVERN.

[Use on all projects. Insert current date of specifications.]

SEE SPECIAL PROVISIONS FOR ALL XXXX.6XX SERIES PAY ITEMS FOR ADDITIONAL REQUIREMENTS.

[Use on all projects.]

THE BAR SIZES SHOWN IN THIS PLAN ARE IN U.S. CUSTOMARY DESIGNATIONS.

[Use on all projects.]

PROVIDE EPOXY COATED BARS MARKED WITH THE SUFFIX "E" IN ACCORDANCE WITH SPEC. 3301.

[Use on all projects.]

PROVIDE STAINLESS STEEL BARS MARKED WITH THE SUFFIX "S" IN ACCORDANCE WITH THE SPECIAL PROVISIONS.

[Use as required.]

PROVIDE EPOXY COATED 4% CHROMIUM BARS MARKED WITH THE SUFFIX "M" IN ACCORDANCE WITH THE SPECIAL PROVISIONS.

[Use as required.]

PROVIDE GFRP BARS MARKED WITH THE SUFFIX "F" IN ACCORDANCE WITH THE SPECIAL PROVISIONS.

[Use as required.]

BRIDGE ELEMENTS WITH SYMBOL (MC) NEXT TO MIX DESIGNATION REQUIRE COMPLIANCE WITH MASS CONCRETE PROVISIONS.

[Use as required. Apply the (MC) symbol to all concrete mix designation callouts depicted in the Final Bridge Plan that require mass concrete.]

THE SUBSURFACE UTILITY INFORMATION IN THIS PLAN IS UTILITY QUALITY LEVEL \_\_\_\_\_. THIS UTILITY QUALITY LEVEL WAS DETERMINED ACCORDING TO THE GUIDELINES OF CI/ASCE 38-02, ENTITLED "STANDARD GUIDELINES FOR THE COLLECTION AND DEPICTION OF EXISTING SUBSURFACE UTILITY DATA".

[Use on all projects. Insert quality level shown in Preliminary Bridge Plan. When unknown, the default level is "D", which is the lowest level.]

**APPENDIX 2-C (Continued)**  
**STANDARD PLAN NOTES**

Use standard notes that are relevant to the project. Text found in brackets [ ] next to a standard note provides guidance on its use and should not be included in the Plan.

**B. CONSTRUCTION NOTES (CONT'D)**

THE GIRDERS HAVE BEEN DESIGNED AND DETAILED WITHOUT DIAPHRAGMS. CONTRACTOR'S ENGINEER TO DESIGN, AND CONTRACTOR TO CONSTRUCT A TEMPORARY BRACING SYSTEM AND/OR A DECK FALSEWORK/FORMWORK SYSTEM. PROVIDE A SYSTEM WITH LATERAL AND ROTATIONAL STABILITY OF THE GIRDERS TO RESIST UNSYMMETRICAL CONCRETE AND CONSTRUCTION LOADS UNTIL THE DECK CONCRETE HAS ATTAINED A MINIMUM STRENGTH OF 2800 PSI.

[Use on all plans with pretensioned RB shapes or 27M, 30MH, or 35MH I-beams where diaphragms are not used.]

INSTALL SETTLEMENT PLATE BEHIND EACH ABUTMENT. SEE GRADING PLANS.

[Use when specified in Foundation Recommendations.]

DO NOT START CONSTRUCTION OF EACH ABUTMENT UNTIL THE APPROACH FILL AT THAT ABUTMENT HAS BEEN CONSTRUCTED TO THE FULL HEIGHT AND CROSS SECTION (AND ALLOWED TO SETTLE FOR \_\_\_\_\_ DAYS).

[Use when specified in Foundation Recommendations. Insert waiting period if required.]

**C. SIGNATURE BLOCK**

APPROVED \_\_\_\_\_

STATE BRIDGE ENGINEER

DATE \_\_\_\_\_

[Use on all projects. Provide signature line in the title block on the General Plan and Elevation sheet.]

**D. DRAINAGE AND EROSION CONTROL**

RESTORE SIDE DITCHES AFTER PLACEMENT OF SLOPE PAVING TO PROVIDE DRAINAGE AS DIRECTED BY THE ENGINEER. INCLUDE RESTORATION COSTS IN PRICE BID FOR STRUCTURE EXCAVATION.

[Use this note on railroad underpasses.]

PLACE \_\_\_\_\_ PIPE UNDER GRADING PORTION OF CONTRACT.

[Use this note with combined Bridge and Roadway contracts only. Modify the notes to suit job requirements. Insert pipe description.]

**APPENDIX 2-C (Continued)**  
**STANDARD PLAN NOTES**

Use standard notes that are relevant to the project. Text found in brackets [ ] next to a standard note provides guidance on its use and should not be included in the Plan.

**E. EXCAVATION AND EARTHWORK**

QUANTITY OF STRUCTURE EXCAVATION FOR PAYMENT IS COMPUTED WITH THE ELEVATION SHOWN FOR EACH SUBSTRUCTURE UNIT AS THE UPPER LIMIT. PAYMENT FOR EXCAVATION ABOVE THESE ELEVATIONS IS UNDER THE GRADING PORTION OF THE CONTRACT.

[Use this note when rock and other type excavation will be encountered. Do not use this note when lump sum payment for structure excavation is used. Specify an elevation for top of exposed or buried rock and add the note "Average elevations of top of rock are assumed for estimated plan quantities."]

THE LOWER LIMITS OF STRUCTURE EXCAVATION CLASS E IS THE SAME AS THE UPPER LIMITS OF STRUCTURE EXCAVATION CLASS WE EXCEPT FOR ROCK EXCAVATIONS.

[Use as required.]

ROADWAY (OR CHANNEL) EXCAVATION WILL BE COMPLETED BY OTHERS IN ADVANCE OF BRIDGE CONSTRUCTION.

[Not applicable on combined project.]

KEY FOOTINGS INTO SOUND BEDROCK AS DIRECTED BY THE ENGINEER. PROVIDE 1'-0" MINIMUM COVER TO TOP OF FOOTINGS.

[Use as required.]

DRESS SLOPES AND PLACE FILTER MATERIALS AND RIPRAP IN APPROXIMATE AREAS AS DIRECTED BY THE ENGINEER.

[Use as required.]

SUBCUT FOOTING A MINIMUM OF 2'-0" AND PLACE AGGREGATE BACKFILL (CONFORMING TO SPEC. 3149.2E) IN ACCORDANCE WITH SPEC. 2451 AS SHOWN. COMPACT TO 100% MAXIMUM DENSITY IN ACCORDANCE WITH SPEC. 2106.3G. SEE SPECIAL PROVISIONS.

[Use when a subcut for a spread footing is required.]

**APPENDIX 2-C (Continued)**  
**STANDARD PLAN NOTES**

Use standard notes that are relevant to the project. Text found in brackets [ ] next to a standard note provides guidance on its use and should not be included in the Plan.

**F. REINFORCEMENT**

## SPIRAL DATA

OUTSIDE DIAMETER \_\_\_\_\_

HEIGHT \_\_\_\_\_

PITCH \_\_\_\_\_

SPIRAL ROD SIZE, PLAIN ROUND \_\_\_\_\_

WEIGHT, EACH \_\_\_\_\_

[Use for round columns that contain spiral reinforcement. Insert appropriate data.]

OUTSIDE DIAMETER OF DOWEL CIRCLE TO BE  $2\frac{1}{4}$ " LESS THAN INSIDE DIAMETER OF SPIRAL.

[Use for round columns that contain spiral reinforcement. Where No. 10 and larger sized column vertical bars are used, increase the  $2\frac{1}{4}$ " dimension where required to provide for a proper fit.]

**APPENDIX 2-C (Continued)**  
**STANDARD PLAN NOTES**

Use standard notes that are relevant to the project. Text found in brackets [ ] next to a standard note provides guidance on its use and should not be included in the Plan.

**G. FOUNDATIONS**

**a. SPREAD FOOTINGS ON SOIL**

<b>_____ ABUTMENT SPREAD FOOTING LOAD DATA</b>	
* _____ † _____ DESIGN BEARING PRESSURE	TONS/SQ FT
EFFECTIVE WIDTH B'	FT
_____ † _____ BEARING RESISTANCE $\phi_b \cdot Q_n$	TONS/SQ FT

\* BASED ON \_\_\_\_\_ LOAD COMBINATION.

<b>PIER _____ SPREAD FOOTING LOAD DATA</b>	
* _____ † _____ DESIGN BEARING PRESSURE	TONS/SQ FT
EFFECTIVE WIDTH B' (PERPENDICULAR TO PIER)	FT
EFFECTIVE LENGTH L' (PARALLEL TO PIER)	FT
_____ † _____ BEARING RESISTANCE $\phi_b \cdot Q_n$	TONS/SQ FT

\* BASED ON \_\_\_\_\_ LOAD COMBINATION.

[Use for spread footing foundations on soil. Insert data. Round loads to nearest 0.1 tons/sq ft.]  
 [ † Insert "SERVICE" or "FACTORED" based on governing limit state.]

**b. SPREAD FOOTINGS ON ROCK**

<b>_____ ABUTMENT SPREAD FOOTING LOAD DATA</b>	
* MAX FACTORED TRAPEZOIDAL DESIGN BEARING PRESSURE	TONS/SQ FT
* MIN FACTORED TRAPEZOIDAL DESIGN BEARING PRESSURE	TONS/SQ FT
FACTORED BEARING RESISTANCE $\phi_b \cdot Q_n$	TONS/SQ FT

\* BASED ON \_\_\_\_\_ LOAD COMBINATION.

<b>PIER _____ SPREAD FOOTING LOAD DATA</b>	
* FTG CORNER 1 MAXIMUM FACTORED TRAPEZOIDAL DESIGN BEARING PRESSURE	TONS/SQ FT
* FTG CORNER 2 FACTORED TRAPEZOIDAL DESIGN BEARING PRESSURE	TONS/SQ FT
* FTG CORNER 3 FACTORED TRAPEZOIDAL DESIGN BEARING PRESSURE	TONS/SQ FT
* FTG CORNER 4 FACTORED TRAPEZOIDAL DESIGN BEARING PRESSURE	TONS/SQ FT
FACTORED BEARING RESISTANCE $\phi_b \cdot Q_n$	TONS/SQ FT

\* BASED ON \_\_\_\_\_ LOAD COMBINATION.

[Use for spread footing foundations on rock. Define footing corners in bridge Plan. Insert data. Round loads to nearest 0.1 tons/sq. ft.]

**APPENDIX 2-C (Continued)**  
**STANDARD PLAN NOTES**

Use standard notes that are relevant to the project. Text found in brackets [ ] next to a standard note provides guidance on its use and should not be included in the Plan.

**G. FOUNDATIONS (CONT'D)**

**c. LOADS FOR PILES**

_____ ABUTMENT COMPUTED PILE LOAD – TONS/PILE	
FACTORED DEAD LOAD + EARTH PRESSURE	
FACTORED LIVE LOAD	
* FACTORED DESIGN LOAD	

\* BASED ON \_\_\_\_\_ LOAD COMBINATION.

PIER _____ COMPUTED PILE LOAD – TONS/PILE	
FACTORED DEAD LOAD	
FACTORED LIVE LOAD	
FACTORED OVERTURNING	
* FACTORED DESIGN LOAD	

\* BASED ON \_\_\_\_\_ LOAD COMBINATION.

[Use for pile foundations. Insert data. Round loads to nearest 0.1 tons/pile.]

**d. ADDITIONAL LOAD TABLE FOR PILES WHERE DOWNDRAG OCCURS**

_____ ABUTMENT COMPUTED PILE LOAD – TONS/PILE	
FACTORED DEAD LOAD + EARTH PRESSURE	
FACTORED DOWNDRAG	
*** FACTORED DEAD LOAD + EARTH PRESSURE + DOWNDRAG	

\*\*\* BASED ON \_\_\_\_\_ LOAD COMBINATION,  
 NOT INCLUDING TRANSIENT LOADS. ONLY USED FOR  
 COMPARISON WITH FACTORED STRUCTURAL  
 RESISTANCE. NOT TO BE USED FOR DRIVING.

PIER _____ COMPUTED PILE LOAD – TONS/PILE	
FACTORED DEAD LOAD	
FACTORED OVERTURNING	
FACTORED DOWNDRAG	
*** FACTORED DEAD LOAD + OVERTURNING + DOWNDRAG	

\*\*\* BASED ON \_\_\_\_\_ LOAD COMBINATION,  
 NOT INCLUDING TRANSIENT LOADS. ONLY USED FOR  
 COMPARISON WITH FACTORED STRUCTURAL  
 RESISTANCE. NOT TO BE USED FOR DRIVING.

[Use for pile foundations where downdrag occurs. Use this table in addition to the standard load table shown above in c. Insert data. Round loads to nearest 0.1 tons/pile.]

**APPENDIX 2-C (Continued)**  
**STANDARD PLAN NOTES**

Use standard notes that are relevant to the project. Text found in brackets [ ] next to a standard note provides guidance on its use and should not be included in the Plan.

**G. FOUNDATIONS (CONT'D)**

**e. H-PILE RESISTANCE**

_____ ABUTMENT REQUIRED NOMINAL PILE BEARING RESISTANCE FOR H-PILES $R_n$ – TONS/PILE		
FIELD CONTROL METHOD	$\phi_{dyn}$	** $R_n$
MnDOT PILE FORMULA 2012 (MPF12) $R_n = 20 \sqrt{\frac{W \times H}{1000}} \times \log\left(\frac{10}{S}\right)$	0.60	
PDA	0.65	

\*\*  $R_n$  = (FACTORED DESIGN LOAD) /  $\phi_{dyn}$

PIER _____ REQUIRED NOMINAL PILE BEARING RESISTANCE FOR H-PILES $R_n$ – TONS/PILE		
FIELD CONTROL METHOD	$\phi_{dyn}$	** $R_n$
MnDOT PILE FORMULA 2012 (MPF12) $R_n = 20 \sqrt{\frac{W \times H}{1000}} \times \log\left(\frac{10}{S}\right)$	0.60	
PDA	0.65	

\*\*  $R_n$  = (FACTORED DESIGN LOAD) /  $\phi_{dyn}$

[Use for H-pile foundations. Insert data. Round loads to nearest 0.1 tons/pile.]

**f. CIP CONCRETE PILE RESISTANCE**

_____ ABUTMENT REQUIRED NOMINAL PILE BEARING RESISTANCE FOR CIP PILES $R_n$ – TONS/PILE		
FIELD CONTROL METHOD	$\phi_{dyn}$	** $R_n$
MnDOT PILE FORMULA 2012 (MPF12) $R_n = 20 \sqrt{\frac{W \times H}{1000}} \times \log\left(\frac{10}{S}\right)$	0.50	
PDA	0.65	

\*\*  $R_n$  = (FACTORED DESIGN LOAD) /  $\phi_{dyn}$

PIER _____ REQUIRED NOMINAL PILE BEARING RESISTANCE FOR CIP PILES $R_n$ – TONS/PILE		
FIELD CONTROL METHOD	$\phi_{dyn}$	** $R_n$
MnDOT PILE FORMULA 2012 (MPF12) $R_n = 20 \sqrt{\frac{W \times H}{1000}} \times \log\left(\frac{10}{S}\right)$	0.50	
PDA	0.65	

\*\*  $R_n$  = (FACTORED DESIGN LOAD) /  $\phi_{dyn}$

[Use for cast-in-place concrete pile foundations. Insert data. Round loads to nearest 0.1 tons/pile.]

**APPENDIX 2-C (Continued)**  
**STANDARD PLAN NOTES**

Use standard notes that are relevant to the project. Text found in brackets [ ] next to a standard note provides guidance on its use and should not be included in the Plan.

**G. FOUNDATIONS (CONT'D)**

**g. OTHER**

PILE NOTES [Use for substructure with test piles]

\_\_\_\_\_ TEST PILES \_\_\_\_\_ FT. LONG  
 \_\_\_\_\_ PILES EST. LENGTH \_\_\_\_\_ FT.  
 \_\_\_\_\_ PILES REQ'D FOR \_\_\_\_\_

[Use for projects with piles. Insert number of piles, pile type, pile length, and substructure name.]

PILE NOTES [Use for substructure without test piles]

\_\_\_\_\_ PILES \_\_\_\_\_ FT. LONG REQ'D. FOR \_\_\_\_\_

[Use for projects with piles. Insert number of piles, pile type, pile length, and substructure name.]

PILE NOTES [substructure with special pay items]

\_\_\_\_\_ PILES EST. LENGTH \_\_\_\_\_ FT. REQ'D FOR \_\_\_\_\_

[Use for projects with piles. Insert number of piles, pile type, pile length, and substructure name.]

**GENERAL PILE NOTES**

PILE SPACING SHOWN IS AT BOTTOM OF \_\_\_\_\_\*.

[Use for projects with piles.]

[\* Insert "FOOTING" or "ABUTMENT"]

PILES MARKED THUS (O-> , H-> ) TO BE BATTERED \_\_\_\_\_ PER FOOT IN DIRECTION SHOWN.

[Use for all battered piling.]

PILES TO BE HP - \_\_\_\_\_.

[Use with all steel H piling.]

PILES TO HAVE A NOMINAL DIAMETER OF \_\_\_\_\_ (AND A THICKNESS OF \_\_\_\_\_").

[Use with all CIP piles. Insert pile diameter and wall thickness when specified in Foundation Recommendations.]

FOR PILE SPLICE DETAILS SEE DETAIL \_\_\_\_\_\*

[Use for projects with piles.]

[\* For CIP piles, insert "B201". For H-piles, insert "B202".]

**APPENDIX 2-C (Continued)**  
**STANDARD PLAN NOTES**

Use standard notes that are relevant to the project. Text found in brackets [ ] next to a standard note provides guidance on its use and should not be included in the Plan.

**G. FOUNDATIONS (CONT'D)**

DRIVE TEST PILES TO ESTIMATED FOUNDATION PILE LENGTH. IF REQUIRED NOMINAL PILE BEARING RESISTANCE IS NOT OBTAINED, WAIT 24 HOURS, AND THEN PERFORM REDRIVE IN ACCORDANCE WITH SPEC. 2452.3D.8. IF REQUIRED NOMINAL PILE BEARING RESISTANCE IS NOT OBTAINED AFTER REDRIVE, CONTINUE DRIVING TO FULL TEST PILE LENGTH.

[Use when pile redriving is specified in Foundation Recommendations.]

DENOTES REACTION PILE. INCLUDE IN ITEM "\_\_\_\_\_ TEST PILE \_\_\_ FT. LONG \_\_\_". WELD ANY PILE SPLICES ON REACTION PILES. SEE SPECIAL PROVISIONS.

[Use for pile load test. Insert pile type, length, and size.]

DENOTES STATIC LOAD TEST PILE. DRIVE TO \_\_\_ FT. LONG. INCLUDE IN ITEM "PILE LOAD TEST TYPE \_\_\_". USE STATIC LOAD TEST PILE AS STRUCTURAL SUPPORT PILE AFTER LOAD TEST IS COMPLETE.

[Use for pile load test. Insert length to drive and pile type.]

DO NOT DRIVE FOUNDATION PILES UNTIL RESULTS OF STATIC LOAD TEST ARE PROVIDED TO THE ENGINEER. THE ENGINEER WILL AUTHORIZE PILE LENGTHS AND CONFIRM PILE DRIVING CRITERIA NO LATER THAN 5 WORKING DAYS AFTER RECEIPT OF LOAD TEST RESULTS.

[Use for pile load test.]

**H. STEEL MATERIALS, FABRICATION, AND ERECTION**

DIMENSIONS SHOWN ARE BASED ON AN AMBIENT TEMPERATURE OF 45°F. FABRICATOR TO ADJUST FOR TEMPERATURE AT FABRICATION.

[Include this note on the framing plan.]

ALL STRUCTURAL STEEL SHALL CONFORM TO SPEC 3309 UNLESS OTHERWISE NOTED.

[Use as required.]

INSTALL SHEAR CONNECTORS ON THE TOP FLANGE OF THE GIRDER IN THE FIELD.

[Use as required.]

THE MAXIMUM RESIDUAL CAMBER IN SPAN \_\_\_\_\_ IS \_\_\_ INCHES AT THE \_\_\_\_\_ POINT OF THE SPAN.

[Include this note on camber diagram sheet when residual camber has been included. Insert span number, residual camber, and location.]

**APPENDIX 2-C (Continued)**  
**STANDARD PLAN NOTES**

Use standard notes that are relevant to the project. Text found in brackets [ ] next to a standard note provides guidance on its use and should not be included in the Plan.

**H. STEEL MATERIALS, FABRICATION, AND ERECTION (CONT'D)**

THERE IS NO RESIDUAL CAMBER IN THESE BEAMS.

[Include this note on camber diagram sheet when residual camber has not been included.]

CHORD LINE IN CAMBER DIAGRAM IS A STRAIGHT LINE FROM END TO END OF BEAM SEGMENT AT \_\_\_\_\_\*\_\_\_\_\_.

[Include this note on camber diagram sheet of steel girder bridges]

[\* For plate girders, insert "TOP OF BEAM WEB". For rolled beams, insert "BOTTOM OF TOP FLANGE".]

CAMBER DIAGRAM SHOWN IS FOR BEAMS IN UNLOADED POSITION AND PROVIDES FOR ALL DEAD LOAD DEFLECTIONS (AND RESIDUAL CAMBER).

[Include this note on camber diagram sheet of steel girder bridges. Include residual camber portion of note when used in the beams.]

LINE ASSEMBLY REQUIRED IN ACCORDANCE WITH SPEC. 2471 FOR BEAM SPLICES.  
LINE ASSEMBLE THE SECTION FROM \_\_\_\_\_ TO \_\_\_\_\_ .

[Use on bridges where girders were designed using line girder analysis. Check with the Structural Metals Unit; abutment to abutment if < 300 ft. Three adjacent points of support if > 300 ft.]

FULL ASSEMBLY REQUIRED IN ACCORDANCE WITH SPEC. 2471.

[The use of full assembly should be considered for extremely complicated curved, superelevated structures (i.e. grid or 3D analysis used for design). Check with the Structural Metals Unit and Fabrication Methods Unit.]

PRIOR TO POURING DECK, SNUG TIGHTEN ONLY BOLTS IN THE END DIAPHRAGMS.  
AFTER POURING ALL DECK CONCRETE, FULLY TIGHTEN END DIAPHRAGM BOLTS.

[Use on all skewed steel bridges.]

PROVIDE OVERSIZED HOLES IN END DIAPHRAGM CONNECTIONS.

[Use on all skewed steel bridges.]

DETAIL AND FABRICATE STEEL MEMBERS FOR A NO-LOAD FIT.

[Use on all steel girder bridges.]

**APPENDIX 2-C (Continued)**  
**STANDARD PLAN NOTES**

Use standard notes that are relevant to the project. Text found in brackets [ ] next to a standard note provides guidance on its use and should not be included in the Plan.

**H. STEEL MATERIALS, FABRICATION, AND ERECTION (CONT'D)**

FOR WELDED FLANGE SPLICES, SEE SPEC 2471.3F.1.a.

[Use drawings instead of note only if different than AASHTO/AWS D1.5 Bridge Welding Code 2020 Fig. 2.7 or 2.8.]

FURNISH WEB PLATES IN AVAILABLE MILL LENGTHS AND WIDTHS WITH A MINIMUM NUMBER OF WEB SPLICES. LOCATION OF SPLICES ARE SUBJECT TO THE APPROVAL OF THE ENGINEER. LOCATE SPLICES A MINIMUM OF 1'-0" FROM STIFFENERS OR FLANGE SPLICES.

[Use on all steel plate girder bridges.]

BEARING STIFFENERS AT ABUTMENTS SHALL BE VERTICAL. BEARING STIFFENERS AT PIERS AND DIAPHRAGM CONNECTION STIFFENERS SHALL BE PERPENDICULAR TO FLANGE. ENDS OF BEAMS SHALL BE VERTICAL.

[Use for grades greater than 3% on plate girder bridges or skews greater than 20°.]

BEARING STIFFENERS, DIAPHRAGM CONNECTION STIFFENERS, AND ENDS OF BEAMS SHALL BE PERPENDICULAR TO FLANGE.

[Use for rolled beams or grades  $\leq 3\%$ .]

ALIGN ROWS OF SHEAR CONNECTORS PARALLEL TO THE TRANSVERSE SLAB REINFORCEMENT BARS.

[Use on all steel bridges with shear connectors.]

SHEAR CONNECTORS TO PROJECT A MINIMUM OF 2" INTO DECK STRUCTURAL SLAB. IN NO CASE SHALL SHEAR CONNECTORS PROJECT CLOSER THAN   \* " TO TOP OF DECK STRUCTURAL SLAB. ENGINEER TO FIELD VERIFY BEAM ELEVATION AND AUTHORIZE CONNECTOR LENGTH.

[Use on all steel bridges with shear connectors.]

[\*Insert "1" for decks with 2" wearing course. Insert "3" for monolithic decks.]

CUT FLANGE PLATES FOR BEAMS TO PROPER CURVATURE.

[Use this note when the minimum radius of curvature requirements of AASHTO LRFD Bridge Construction Specifications, Article 11.4.12.2.2 are not met.]

MAKE ALL BOLTED CONNECTIONS WITH  $\frac{7}{8}$ " DIAMETER A325 BOLTS, EXCEPT AS NOTED.

[Use as required.]

**APPENDIX 2-C (Continued)**  
**STANDARD PLAN NOTES**

Use standard notes that are relevant to the project. Text found in brackets [ ] next to a standard note provides guidance on its use and should not be included in the Plan.

**H. STEEL MATERIALS, FABRICATION, AND ERECTION (CONT'D)**

ELEVATIONS SHOWN AT FIELD SPLICES ARE THEORETICAL ELEVATIONS FURNISHED AS A GUIDE FOR ERECTION PURPOSES. ELEVATIONS ARE GIVEN AT TOP OF TOP FLANGE SPLICE PLATE AND INCLUDE DEFLECTIONS FROM WEIGHT OF BEAM AND DIAPHRAGM.

[Use as required.]

DEFLECTIONS SHOWN ARE FOR WEIGHT OF SLAB, CONCRETE OVERLAY, \_\_\_\_\_. NEGATIVE SIGN INDICATES UPLIFT.

[Use as required. Insert barrier, parapet, metal railing, sidewalk, median, etc. as needed. Do not include the weight of steel beams or future wearing course.]

SHEAR CONNECTORS TO CONFORM TO SPEC. 3391.

[Use on all steel bridges with shear connectors.]

**I. CONCRETE PLACEMENTS**

REMOVE ALL NON-GALVANIZED AND NON-EPOXY COATED FERROUS METAL, EXCLUDING SHEAR CONNECTORS, TO WITHIN ½" OF THE TOP FLANGE PRIOR TO CASTING THE DECK.

[Use on all deck-on-beam type bridges. Locate note on the Transverse Section Deck Reinforcement sheet in the bridge Plan.]

MAKE ¾" WIDE x 1" DEEP SAW CUT IN STRUCTURAL SLAB (AND CONCRETE WEARING COURSE) OVER CENTERLINE OF PIERS AS SOON AS THE CUTTING CAN BE DONE WITHOUT RAVELING THE CONCRETE. APPLY TYPE B POLYSTYRENE TO TIPS OF FLANGES THAT PROJECT PAST CENTERLINE OF PIER. SEAL JOINT IN ACCORDANCE WITH SPEC. 3725.

[Use on prestressed concrete beam bridges with double diaphragms and slab continuous over piers. Saw cut both structural slab and concrete wearing course. See Figure 9.2.1.10 in this manual for detail.]

TOP OF SLAB UNDER BARRIER IS LEVEL. THE BOTTOM OF SLAB CONTINUES AT THE SAME SLOPE AS THE ROADWAY.

[Use for slab type bridges. Include note on superstructure sheet that contains cross-section.]

CAST COUNTERWEIGHT AT LEAST 48 HOURS IN ADVANCE OF PLACING DECK SLAB.

[Use as required.]

**APPENDIX 2-C (Continued)**  
**STANDARD PLAN NOTES**

Use standard notes that are relevant to the project. Text found in brackets [ ] next to a standard note provides guidance on its use and should not be included in the Plan.

**I. CONCRETE PLACEMENTS (CONT'D)**

CONSTRUCT BARRIER SUCH THAT ANGLE BETWEEN ROADWAY SURFACE AND BACK FACE OF BARRIER IS NOT GREATER THAN 90°.

[Use for barrier on the high side of bridges with superelevation.]

REQUIRE A MINIMUM OF 72 HOURS BETWEEN ADJACENT DECK POURS.

[Use on all bridges that require a concrete deck pour sequence with two or more separate pours.]

**J. WELDED STEEL BEARING ASSEMBLIES**

STRUCTURAL STEEL SHALL CONFORM TO SPEC. 3306 EXCEPT AS NOTED.

[Use as required.]

INCLUDE SHIMS IN PRICE BID FOR BEARING ASSEMBLIES.

[Add to B-Detail if shims are used.]

PINS AND ROLLERS SHALL CONFORM TO SPEC. 2471.3D.4.

[Use as required.]

PINS SHALL BE COLD FINISHED ALLOY BAR STEEL IN ACCORDANCE WITH SPEC. 3314 TYPE II.

[For pins 5" or less where pin is not made from a larger diameter stock.]

PINS SHALL BE HOT ROLLED ALLOY BAR STEEL IN ACCORDANCE WITH SPEC. 3313 TYPE II.

[For pins over 5" where pin will be made from a larger diameter stock.]

PINTLES SHALL CONFORM TO SPEC. 3309.

[Use as required.]

LUBRICATED BRONZE BUSHINGS SHALL CONFORM TO SPEC. 3329.

[Use as required.]

ANNEAL ALL WELDED BEARING ASSEMBLIES AFTER WELDING. FINISH PIN HOLES AND TOP AND BOTTOM PLATES AFTER ANNEALING.

[For welded rockers and bolsters.]

**APPENDIX 2-C (Continued)**  
**STANDARD PLAN NOTES**

Use standard notes that are relevant to the project. Text found in brackets [ ] next to a standard note provides guidance on its use and should not be included in the Plan.

**J. WELDED STEEL BEARING ASSEMBLIES (CONT'D)**

COAT PINS AND PIN HOLES IN THE SHOP WITH AN APPROVED HEAVY PROTECTIVE GREASE. PRIOR TO ERECTION, CLEAN AND COAT THE PINS AND PIN HOLES WITH AN APPROVED GREASE.

[Use as required.]

PROVIDE A TEMPLATE DEMONSTRATING THAT ANCHOR RODS WILL HAVE 2 INCHES CLEAR DISTANCE TO ALL REINFORCEMENT. REFER TO SPEC. 2472.3C.1 FOR MORE DETAILED INSTRUCTION.

[Include this note with anchor rod placement details.]

\_\_\_ DENOTES ELASTOMERIC BEARING PAD, TYPE \_\_\_. SEE DETAIL B305.

\_\_\_ DENOTES FIXED CURVED PLATE BRG. ASSEMBLY, TYPE \_\_\_. SEE DETAIL B310.

\_\_\_ DENOTES EXPANSION CURVED PLATE BRG. ASSEMBLY, TYPE \_\_\_. SEE DETAIL B311.

[Include appropriate notes(s) on framing plan when elastomeric bearing pads are used. Insert bearing designation and type number.]

PLAN BRIDGE SEAT ELEVATIONS ARE BASED ON AN ASSUMED DISC BEARING HEIGHT OF \_\_\_. DETERMINE FINAL BRIDGE SEAT ELEVATIONS BASED ON ACTUAL HEIGHT OF DISC BEARING ASSEMBLIES. MAKE ANY REQUIRED ADJUSTMENTS TO THE BRIDGE SEAT ELEVATIONS AT NO COST TO THE DEPARTMENT. BRIDGE SEAT PEDESTALS SHALL NOT BE LESS THAN 3" TALL.

[To be used when disc bearings are provided. Include this note on substructure plan sheet where bridge seat elevations are shown. Insert estimated height of disc bearing assembly.]

**K. CUTTING AND REMOVAL OF OLD CONCRETE**

HATCHED AREAS INDICATE CONCRETE REMOVAL.

[Use as required.]

OUTLINE CUTTING LIMITS AND RECEIVE APPROVAL BY THE ENGINEER PRIOR TO ANY CUTTING. PERFORM REMOVAL AND RECONSTRUCTION IN ACCORDANCE WITH SPEC. 2433.

[Use as required.]

**APPENDIX 2-C (Continued)**  
**STANDARD PLAN NOTES**

Use standard notes that are relevant to the project. Text found in brackets [ ] next to a standard note provides guidance on its use and should not be included in the Plan.

**L. JOINTS AND JOINT SEALER**

PLACE CONTROL JOINT IN SIDEWALK (AND MEDIAN) OVER CENTERLINE OF PIER. USE A 1" DEEP BY 3/8" WIDE V SHAPED JOINT IN TOP OF SIDEWALK (AND MEDIAN) AND FINISH WITH 1/4" RADIUS EDGER. USE 1/2" V STRIP IN VERTICAL EDGE OF SIDEWALK. BREAK BOND AT JOINT BY APPROVED METHOD. SEAL CONTROL JOINT IN ACCORDANCE WITH SPEC. 3722. NO REINFORCEMENT THROUGH JOINT.

[Use for raised sidewalk and median concrete sections less than 12" in height.]

INCLUDE 1½" TYPE B POLYSTYRENE BETWEEN APPROACH PANEL AND WINGWALL IN GRADING PLAN.

[Use as required.]

SUPERSTRUCTURE DIMENSIONS ARE BASED ON THE EXPANSION JOINT DIMENSION AT 90° F.

[Use when superstructure details show more than one expansion joint dimension for different temperatures.]

**M. TIMBER BRIDGES**

CONSTRUCTION REQUIREMENTS IN ACCORDANCE WITH SPEC. 2403.

[Use as required.]

FURNISH ALL TIMBER PILING IN ACCORDANCE WITH SPEC. 3471.  
GALVANIZE ALL HARDWARE IN ACCORDANCE WITH SPEC. 3392.  
UPSET THREADS ON ALL BOLTS AFTER INSTALLATION.

[Use as required.]

FURNISH ALL WOOD AS SHOWN IN THE BILL OF MATERIALS.

[Use as required.]

SHAPE TOP OF WING PILE WHICH PROJECTS OUTSIDE OF WING CAP TO A 45° SLOPE.

[Use as required.]

TREAT TOPS OF WING PILES IN ACCORDANCE WITH SPEC. 2403.3E. SEE SPEC. 3491 FOR PRESERVATION REQUIREMENTS.

[Use as required.]

DO NOT PLACE FILL IN BACK OF ABUTMENT UNTIL AFTER SUPERSTRUCTURE IS COMPLETE.

[Use as required.]

**APPENDIX 2-C (Continued)**  
**STANDARD PLAN NOTES**

Use standard notes that are relevant to the project. Text found in brackets [ ] next to a standard note provides guidance on its use and should not be included in the Plan.

**M. TIMBER BRIDGES (CONT'D)**

FASTEN BACKING TO ABUTMENT PILES WITH TWO 60D NAILS AT EACH INTERSECTION.

[Use as required.]

CUT OFF BOLT PROJECTIONS EXCEEDING 1". REPAIR END OF BOLT BY PAINTING WITH AN APPROVED ZINC-RICH PRIMER.

[Use as required.]

INSTALL TIMBER WINGWALL PILES TO THE LENGTH SHOWN IN THE PLANS.

[Use as required.]

PRESERVATIVE TREAT ALL WOOD IN ACCORDANCE WITH SPEC. 3491.

[Use as required.]

FIELD TREAT ALL WOOD THAT IS CUT OR DRILLED IN THE FIELD IN ACCORDANCE WITH SPEC. 2403.3E.

[Use as required.]

ALL PLANKS FOR PREFAB PANELS SHALL BE DOUGLAS FIR-LARCH GRADE \_\_\_\_ (F<sub>bo</sub> = \_\_\_\_ KSI).

[Use as required. Insert grade and reference design value.]

FOR GLULAM RAIL ELEMENTS SEE PLAN SHEET \_\_\_\_ .

[Use as required. Insert sheet number.]

RAIL POSTS, CURBS, SCUPPERS, AND RAIL SPACER BLOCKS SHALL BE DOUGLAS FIR-LARCH GRADE \_\_\_\_ (MIN. F<sub>bo</sub> = \_\_\_\_ KSI).

[Use as required. Insert grade and reference design value.]

ABUTMENT AND PIER CAPS SHALL BE DOUGLAS FIR-LARCH GRADE \_\_\_\_ POSTS AND TIMBERS (F<sub>bo</sub> = \_\_\_\_ KSI). ALL OTHER LUMBER SHALL HAVE MIN. F<sub>bo</sub> = \_\_\_\_ KSI.

[Use as required. Insert grade and reference design values.]

SHOP PAINT PILE PLATE ASSEMBLY AFTER FABRICATION IS COMPLETED. PAINT PIER PILES, PLATES AND RINGS IN ACCORDANCE WITH SPEC. 2452.3J.1 CLEAN AND SPOT COAT AREAS OF PILES, PLATES AND RINGS BURNED DURING WELDING WITH ZINC-RICH PRIMER BEFORE FIELD PAINTING IS STARTED.

[Use as required.]

**APPENDIX 2-C (Continued)**  
**STANDARD PLAN NOTES**

Use standard notes that are relevant to the project. Text found in brackets [ ] next to a standard note provides guidance on its use and should not be included in the Plan.

**M. TIMBER BRIDGES (CONT'D)**

ALL STRUCTURAL STEEL SHALL CONFORM TO SPEC 3306 UNLESS OTHERWISE NOTED.

[Use as required.]

GALVANIZE ALL STRUCTURAL STEEL IN ACCORDANCE WITH SPEC. 3394.

[Use as required.]

**N. MISCELLANEOUS**

F.F. DENOTES FRONT FACE.

B.F. DENOTES BACK FACE.

E.F. DENOTES EACH FACE.

[Use as required.]

TAKE FIELD MEASUREMENTS AS NECESSARY PRIOR TO FABRICATION OF THE \_\_\_\_\_ TO ASSURE PROPER FIT IN THE FINAL WORK.

[Use when not otherwise referenced to Spec. 2433. Insert component to be fabricated.]

BEAM LENGTH DIMENSIONS ARE SLOPED LENGTHS.

[Use where necessary for proper fit for prestressed beams.]

USE GREASE FROM THE "APPROVED/QUALIFIED PRODUCT LIST FOR BRIDGE PRODUCTS, BRIDGE GREASE".

[Use for dowel bar assemblies.]

EPOXY COAT ALL MATERIAL. INCLUDE MATERIAL AND PLACEMENT IN PRICE BID FOR "DOWEL BAR ASSEMBLY (EPOXY COATED)".

[Use for dowel bar assemblies.]

\_\_\_\_\_ PASSAGE BENCH AT EL. \_\_\_\_\_. SURFACE WITH COARSE FILTER AGGREGATE IN ACCORDANCE WITH SPEC. 3149.2H TO PROVIDE SMOOTH SURFACE. INCLUDED IN ITEM "RANDOM RIPRAP CLASS \_\_\_\_\_".

[Use when a passage bench is required. Insert appropriate values.]

"X" DENOTES X END OF BEAM.

[Include on framing plan for pretensioned concrete beam bridges.]

**APPENDIX 2-C (Continued)****STANDARD PLAN NOTES**

Use standard notes that are relevant to the project. Text found in brackets [ ] next to a standard note provides guidance on its use and should not be included in the Plan.

**N. MISCELLANEOUS (CONT'D)**

PROVIDE 1/8 INCH 60 DUROMETER PLAIN ELASTOMERIC PAD OR PREFORMED FABRIC PAD MEETING AASHTO LRFD BRIDGE CONSTRUCTION SPECIFICATION ARTICLE 18.10. WAIVE THE SAMPLING AND TESTING REQUIREMENTS UNDER SPEC 3741, "ELASTOMERIC BEARING PADS," AND AASHTO M251.

[Use as required.]

**APPENDIX 2-D****STANDARD SUMMARY OF QUANTITIES NOTES**

Use standard notes that are relevant to the project. Text found in brackets [ ] next to a standard note provides guidance on its use and should not be included in the Plan.

PAYMENT FOR \_\_\_\_\_ INCLUDED IN ITEM "\_\_\_\_\_".

[For minor items with small quantities that are not listed as pay items (joint filler, nameplate, etc.). Fill in blanks with description and appropriate pay item (e.g. for joint filler: Payment for joint filler included in item "Structural Concrete (3B52)").]

INCLUDED IN WEIGHT OF "STRUCTURAL STEEL (33\_\_)".

[Miscellaneous steel quantities (protection angle, etc.).]

DOES NOT INCLUDE TEST PILES.

[Use when piling quantities are listed.]

INCLUDES SLAB, END DIAPHRAGM, MEDIAN BARRIER, SIDEWALK, AND RAILING REINFORCEMENT.

[Add to epoxy coated reinforcement bar totals. Remove components that do not apply.]

"CONCRETE WEARING COURSE (3U17A)" INCLUDES \_\_\_\_\_ SQUARE FEET FOR BRIDGE APPROACH PANELS.

[Use when the item as listed in the Summary of Quantities for Superstructure is paid for on a square foot basis. Insert area.]

PAYMENT FOR BEARINGS INCLUDED IN ITEM "BEARING ASSEMBLY" PER EACH.

[Use as required.]

PAYMENT FOR ANCHORAGES INCLUDED IN ITEM "ANCHORAGES TYPE REINF BARS" PER EACH.

[Use as required.]

PAYMENT FOR ANCHORAGES INCLUDED IN ITEM "GROUTED REINFORCEMENT BARS" PER EACH.

[Use as required.]

PAYMENT FOR THREADED COUPLERS INCLUDED IN ITEM "COUPLERS (REINFORCEMENT BARS) T-\_\_\_" PER EACH.

[Use as required. Specify bar size.]

MEMBRANE WATERPROOFING SYSTEM IN ACCORDANCE WITH SPEC 2481.3B TO BE INCLUDED IN ITEM "STRUCTURAL CONCRETE (\_\_\_\_)".

[Use as required. Insert concrete mix.]

**APPENDIX 2-D (Continued)****STANDARD SUMMARY OF QUANTITIES NOTES**

Use standard notes that are relevant to the project. Text found in brackets [ ] next to a standard note provides guidance on its use and should not be included in the Plan.

PAYMENT FOR SHEAR CONNECTORS INCLUDED IN ITEM "SHEAR STUDS".

[Use as required.]

SEE DRAINAGE SYSTEM TYPE (B910).

[Use as required.]

INCLUDE PILE REDRIVING AND PILE ANALYSIS FOR THE REACTION PILES AND STATIC LOAD TEST PILE IN PRICE BID FOR "PILE LOAD TEST TYPE \_\_\_".

[Use as required. Insert type number.]

APPENDIX 2-E  
CONVERSION FROM INCHES TO DECIMALS OF A FOOT

	0"	1"	2"	3"	4"	5"		6"	7"	8"	9"	10"	11"	
0	.0000	.0833	.1667	.2500	.3333	.4167	0	.5000	.5833	.6667	.7500	.8333	.9167	0
1/16	.0052	.0885	.1719	.2552	.3385	.4219	1/16	.5052	.5885	.6719	.7552	.8385	.9219	1/16
1/8	.0104	.0938	.1771	.2604	.3438	.4271	1/8	.5104	.5938	.6771	.7604	.8438	.9271	1/8
3/16	.0156	.0990	.1823	.2656	.3490	.4323	3/16	.5156	.5990	.6823	.7656	.8490	.9323	3/16
1/4	.0208	.1042	.1875	.2708	.3542	.4375	1/4	.5208	.6042	.6875	.7708	.8542	.9375	1/4
5/16	.0260	.1094	.1927	.2760	.3594	.4427	5/16	.5260	.6094	.6927	.7760	.8594	.9427	5/16
3/8	.0313	.1146	.1979	.2813	.3646	.4479	3/8	.5313	.6146	.6979	.7813	.8646	.9479	3/8
7/16	.0365	.1198	.2032	.2865	.3698	.4531	7/16	.5365	.6198	.7032	.7865	.8698	.9531	7/16
1/2	.0417	.1250	.2083	.2917	.3750	.4583	1/2	.5417	.6250	.7083	.7917	.8750	.9583	1/2
9/16	.0469	.1302	.2135	.2969	.3802	.4635	9/16	.5469	.6302	.7135	.7969	.8802	.9635	9/16
5/8	.0521	.1354	.2188	.3021	.3854	.4688	5/8	.5521	.6354	.7188	.8021	.8854	.9688	5/8
11/16	.0573	.1406	.2240	.3073	.3906	.4740	11/16	.5573	.6406	.7240	.8073	.8906	.9740	11/16
3/4	.0625	.1458	.2292	.3125	.3958	.4792	3/4	.5625	.6458	.7292	.8125	.8958	.9792	3/4
13/16	.0677	.1510	.2344	.3177	.4010	.4844	13/16	.5677	.6510	.7344	.8177	.9010	.9844	13/16
7/8	.0729	.1563	.2396	.3229	.4063	.4896	7/8	.5729	.6563	.7396	.8229	.9063	.9896	7/8
15/16	.0781	.1615	.2448	.3281	.4115	.4948	15/16	.5781	.6615	.7448	.8281	.9115	.9948	15/16
	0"	1"	2"	3"	4"	5"		6"	7"	8"	9"	10"	11"	

**CONVERSION TABLE  
INCHES TO DECIMALS OF A FOOT**

**[ This Page Intentionally Left Blank ]**